

November 14, 2025

Mr. Sarwan Signh

Royal Truck and Trailer Sales Ltd 73 Stafford Drive Brampton, ON L6W 1L3

Re: Proposed Warehouse Development

3027 Cedar Creek Road (RR 97), Township of North Dumfries, Waterloo

Region

**Transportation Study** 

CGE Transportation Consulting is pleased to submit this Transportation Study for the proposed warehouse development, located at 3027 Cedar Creek Road (RR 97) in the Township of North Dumfries, Waterloo Region.

The proposal includes a 107,639-square-foot warehouse and logistics facility (truck terminal) featuring 24 docking bays (19 loading, 5 repair). The development will include 33 employee parking spaces, 233 trailer parking spaces, and 39 tractor parking spaces. Two access points from Cedar Creek Road (RR 97) will flank the warehouse. To the immediate west of the proposed warehouse building, a 9300 square metre Stormwater Management Facility is proposed.

The study demonstrates that the development will have minimal impact on the surrounding road network, and the access connections are projected to operate at an acceptable level of service with minimal delays.

Should you have any questions regarding this study, please do not hesitate to contact the undersigned.

Yours truly,

**CGE TRANSPORTATION CONSULTING** 

Casey Ge, P.Eng.

President

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# 1.0 Introduction

CGE Transportation Consulting was retained by Royal Truck and Trailer Sales Ltd. to prepare a Transportation Study for a proposed warehouse development located at 3027 Cedar Creek Road (RR 97) in the Town of North Dumfries, Waterloo Region.

### **Existing Site Description:**

The site is bounded by Cedar Creek Road (RR 97) to the north, Highway 401 to the south, industrial developments to the east, and agricultural lands to the west. Currently vacant, the subject site is zoned as "Z.1 – Agricultural". The location of the proposed development is illustrated in **Figure 1**.

### **Development Proposal Description:**

The proposal includes a 107,639-square-foot warehouse and logistics facility with 24 docking bays (19 loading, 5 repair). A stormwater management facility is also proposed. The development will also include 33 employee parking spaces, 233 trailer parking spaces, and 39 tractor parking spaces.

Discussions with the owner have indicated that the trailers will follow a weekly usage pattern. This means trailers will be stationed on-site for extended periods, rather than being subject to daily turnover. Such an operational model is consistent with standard warehousing practices, where the frequency of loading and unloading activities is typically on a weekly basis. Consequently, the high number of trailer parking spaces should not be interpreted as correlating to an equivalent number of daily trips. Instead, the spaces are intended to accommodate the storage requirements inherent in logistical operations. The warehouse is projected to employ approximately 6 staff members, while the office will house between 12 and 16 employees.

The site design includes two access points from Cedar Creek Road (RR 97), located on either side of the proposed warehouse building. The full build-out of this project is targeted for completion in 2029. The Concept Plan of Subdivision is provided in **Figure 2**.

### Scope of Work:

Pre-consultation meetings indicate that the Township of North Dumfries requires a Traffic Impact Study for this application.

The study area consists of the following intersections:

- Cedar Creek Road (RR 97) / East Site Access
- Cedar Creek Road (RR 97) / West Site Access

The study analyzed the following scenarios:

- 2024 Existing Conditions
- Future Background 2029 Conditions
- Full Build 2029 Conditions

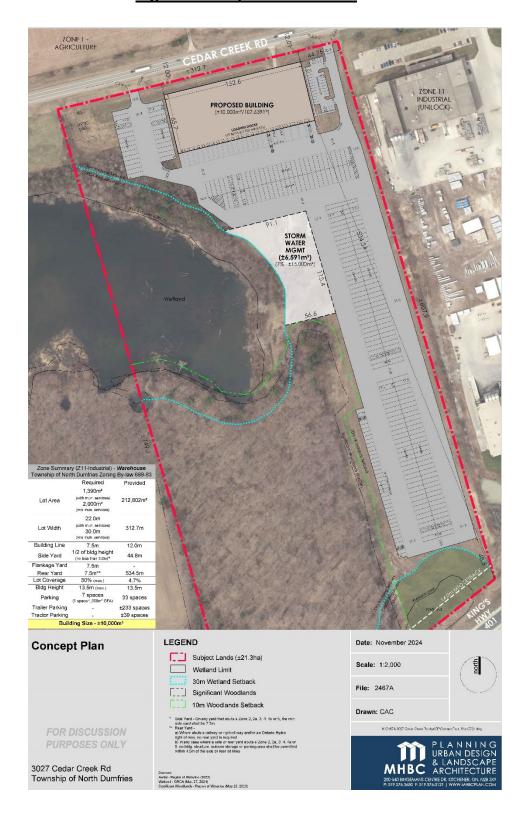
The analysis has been conducted for both weekday AM and PM peak hours.

# Figure 1 Site Location



Source: Waterloo Region GIS Maps

# Figure 2 Proposed Site Plan



### 2.0 Existing conditions

# 2.1 Existing Roadway Conditions

Cedar Creek Road (RR 97) is a two-lane, east-west arterial roadway with an 80 km/h speed limit, under the Region's control. It has paved shoulders on both sides but does not provide sidewalks or on-street parking, typical of a rural road cross-section.

A review of the Region's current and future construction area does not show any immediate projects planned along Cedar Creek Road (RR 97) in the vicinity of the subject site.

The proposed development features two full access connections to Cedar Creek Road (RR 97).

#### 2.2 Traffic Volumes

Turning movement counts at the adjacent intersections of Cedar Creek Road (RR 97) & Northumberland Street and Cedar Creek Road (RR 97) & Hwy 401 WB Ramp/Driveway were used to determine the volumes along Cedar Creek Road (RR 97) at the proposed access connections. Traffic data collection for these intersections was performed on June 1, 2022 can be found in **Appendix A**, and is summarized below:

Table 1 2022 Traffic Volumes

Intersection	Time Period	EB (vph)	WB (vph)
Cedar Creek Road (RR 97) &	AM Peak Hour	466	311
Northumberland Street	PM Peak Hour	355	639
Cedar Creek Road (RR 97) & Hwy 401	AM Peak Hour	490	343
WB Ramp/Driveway	PM Peak Hour	398	617
Highest Observed Volumes	AM Peak Hour	490	343
	PM Peak Hour	398	639

Based on the foregoing, for a conservative analysis, the highest observed eastbound and westbound volumes were used for the study. However, it is noted that the data collection was performed in 2022. Typically, counts that are no more than 3 years old represent existing conditions. However, for a conservative analysis, the volumes were grown by 2% per annum, which is the approximate annual growth rate based on the Waterloo Region forecasts noted in the *Moving Forward – 2018 Transportation Master Plan*. Based on that, the projected volumes for 2024 are:

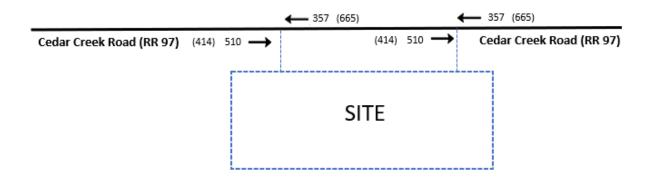
Table 2 2024 Traffic Volumes

Year	Time Period	Entering EB (vph)	Exiting WB (vph)
2024	AM Peak Hour	510	357
	PM Peak Hour	414	665

Figure 3 displays existing traffic volume.

Figure 3 Existing Traffic Volumes





# Legend

1↑↑ Lane Configuration XX(XX) AM(PM) Peak Volumes

# 3.0 METHODOLOGY

# 3.1 Base Assumptions

Intersection capacity analysis was conducted using Synchro v11.0. Trip generation for the proposed development was calculated using the 11th edition of the Institute of Transportation Engineers (ITE) *Trip Generation Manual*.

The input parameters for Synchro, used in conducting the intersection capacity analysis, were based on the recommendations contained in the Region's *Transportation Impact Studies (TIS) Requirements for Capacity Analysis, Roundabouts, Signal Warrants.* 

Turn lane requirements on Cedar Creek Road (RR 97) were examined using the recommendations in the Regions' *Transportation Impact Study Guidelines*.

# **3.2 Background Developments**

In the site's vicinity, two background developments were identified and included in the study. Detailed site-specific trip information is provided in **Appendix B**. The studies referenced include:

- 2804 Cedar Creek Road Industrial Subdivision Traffic Impact Assessment.
- Regional Roads 97 and 47 Industrial Lands Traffic Impact Study.

# 3.3 Background Growth

Volumes were increased by 2% to estimate background growth for Full Build 2029 conditions. This growth rate is consistent with the background developments reports as well as the approximate annual growth rate based on the Waterloo Region forecasts noted in the *Moving Forward – 2018 Transportation Master Plan*.

# **3.4 Site Trip Generation**

The proposal includes a 107,639-square-foot warehouse and logistics facility with 24 docking bays. The development will also include 33 employee parking spaces, 242 trailer parking spaces, and 39 tractor parking spaces.

The weekly trailer usage pattern, common in the industry, minimizes daily truck movements, underscoring that the high number of trailer spaces is for storage, not daily trips. The facility will staff approximately 6 warehouse personnel and 12-16 office staff (22 employees in total).

The *ITE Trip Generation Man*ual, 11th Edition Land Use Code 150 (Warehousing), was used. This code provides empirically-derived data specific to warehousing activities, accounting for factors such as gross floor area, number of employees, and typical operational patterns. The trip generation was calculated using whichever method yields the greater number of trips:

- Independent Variable: Employees (22 employees)
  - Vehicle Trip Generation
  - Truck Trip Generation
- Independent Variable: 1,000 Sq. Ft. GFA (107,639 Sq. Ft.)
  - Vehicle Trip Generation
  - Truck Trip Generation

**Table 3** contains the summary of the land uses and sizes used for the trip generation estimates. The trip generation graphs, and detailed calculations can be found in **Appendix C**.

**Table 3 Estimated Traffic Generation** 

	Land Use				AM Peak Hour			PM Peak Hour		
Land Use	ITE Code	Independent Variable	Size		Enter	Exit	Total	Enter	Exit	Total
	150	GFA (Vehicle)	108	1000 Sq. Ft.	28	9	37	11	28	39
		GFA (Truck)	100	GFA	1	1	2	2	1	3
Marabausing		Total Volume		29	10	39	13	29	42	
Warehousing		Employees (Vehicle)	22	Employees	12	4	16	5	10	15
		Employees (Truck)	22	Employees	1	1	2	1	1	2
		Total Vo	olume		13	5	18	6	11	17

Trip generation estimates indicate that using Gross Floor Area (GFA) as an independent variable correlates with higher vehicle and truck volumes. To account for this, the study conservatively estimated 37 and 39 new two-way vehicle trips, and 2 and 3 new two-way truck trips, to the adjacent network during weekday AM and PM peak hours, respectively.

# 3.5 Trip Distribution

Trips for this proposed development were assigned to the surrounding roadway network using engineering judgment and existing traffic patterns. The proposed trip distribution for this project is:

### **AM Peak Hour Inbound:**

- 40% to/from east on Cedar Creek Road (RR 97)
- 60% to/from west on Cedar Creek Road (RR 97)

#### AM Peak Hour Outbound:

- 60% to/from east on Cedar Creek Road (RR 97)
- 40% to/from west on Cedar Creek Road (RR 97)

#### PM Peak Hour Inbound:

- 50% to/from east on Cedar Creek Road (RR 97)
- 50% to/from west on Cedar Creek Road (RR 97)

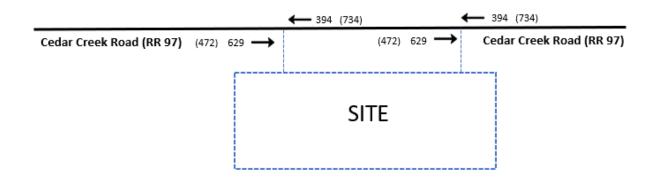
#### PM Peak Hour Outbound:

- 50% to/from east on Cedar Creek Road (RR 97)
- 50% to/from west on Cedar Creek Road (RR 97)

Future Background volumes for 2029 are shown in **Figure 4.** The project site trips are shown in **Figure 5.** Full Build 2029 volumes are shown in **Figure 6.** 

Figure 4 Future Background 2029 Volumes



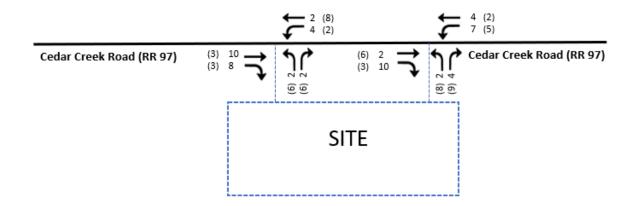


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「↑↑ Lane Configuration XX(XX) AM(PM) Peak Volumes

Figure 5 Project Site Trips



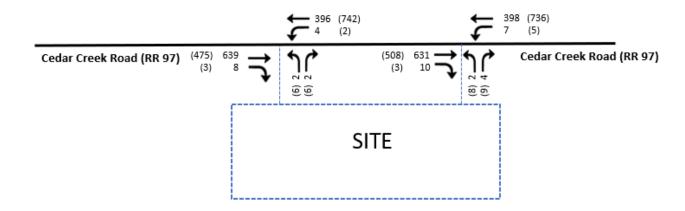


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「 Lane Configuration XX(XX) AM(PM) Peak Volumes

# Figure 6 Full Build 2029 Volumes





# Legend

1个 Lane Configuration XX(XX) AM(PM) Peak Volumes

# 4.0 TURN LANE/ACCCESS MANAGEMENT

# 4.1 Left-Turn Lane Cedar Creek Road (RR 97)

The Region's Traffic Impact Study (TIS) guidelines recommend using the Ministry of Transportation (MTO) Geometric Design Guidelines to determine the need for left-turn lanes at unsignalized intersections. These guidelines offer graphical thresholds/warrants based on design speed, advancing traffic, oncoming traffic, and percentage of left turns. **Table 4** presents the Full Build 2029 volumes used in the analysis, with corresponding graphs available in **Appendix D**.

Table 4 Left-Turn Lane Analysis

Driveway	AM/ PM	Approach	Design Speed	Advancing Vol	Opposing Vol	LT Vol	Turn Lane needed?
Cedar Creek Rd	AM			405	641	7	Yes
(RR 97) & East Site Access	PM	WB	00	741	511	5	Yes
Cedar Creek Rd	AM	VVD	90	400	647	4	Yes
(RR 97) & West Site Access	PM			744	478	2	Yes

Based on Full Build 2029 volumes, a westbound left-turn lane from Cedar Creek Road (RR 97) is required for the project accesses. It is **recommended to construct a westbound left-turn lane** at the site access connections to Cedar Creek Road (RR 97).

# 4.2 Right-Turn Lane on Cedar Creek Road (RR97)

The Region recommends considering right-turn lanes at unsignalized intersections based on level of service (LOS) and operating conditions, as outlined in the Traffic Impact Study (TIS) guidelines. **Table 5** below provides a summary of the LOS at Full Build 2029 Conditions at the access connections to Cedar Creek Road (RR 97).

Table 5 Site Access - Intersection Analysis Summary

Intersection LOS, Delay, and Queue by Movement - 2029 Future Total											
Intersection	Approach	Movement		AM			PM				
Intersection	Approach	wovernent	LOS	Delay	Queue	LOS	Delay	Queue			
	EB	TH		Free			Free				
Cedar Creek Rd & East Site Access	ĽÐ	RT		riee			riee				
	WB	LT	Α	8.8	-	Α	8.7	-			
	VVD	TH	Free		Free						
	NB	LT	С	15.4	2 m	С	18.2	3 m			
	EB	TH	Free			Free					
Cedar Creek Rd & West	ĽĎ	RT		riee			riee				
Site Access	WB	LT	Α	8.8	-	Α	8.7	-			
Site Access	VVD	TH		Free		Free					
	NB	LT	С	15.4	2 m	С	18.2	3 m			

Analysis shows that the access will operate with excellent LOS for all movements, no eastbound right-turn lanes are required.

# 4.3 Sight Distance

Minimum sight distance requirements were evaluated based on the guidelines provided in the Transportation Association of Canada's *Geometric Design Guide for Canadian Roads, Chapter 9, Intersections (2017)*. The sight distance evaluation was conducted using a design speed of posted speed + 10 km/h.

	ISD = $0.278 V_{ma}$	jor t <sub>g</sub>	(9.9.1)	
Where:				
ISD =	intersection sigh	t distance (le	ngth of the leg	
	of sight triangle	along the ma	jor road) (m)	
V <sub>major</sub> =	design speed of	the major roa	id (km/h)	
t <sub>e</sub> =	time gap for min	or road vehic	le to enter the	
-8	major road (s)			
	major road (3)			
Design V	ehicle		ne Gap $(t_g)(s)$ at	
Design v	cincic	Design:	Speed of Major Road	
Passenger car		7.5		
Single-unit truck		9.5		

The calculated intersection sight distance using the above formula and parameters results in the following: Combination Truck (WB 19 and WB 20): 0.278\*90\*11.5 = 287.73 metres

Table 6 Sightline Distance Review at Cedar Creek Road (RR 97)

Intersection	Decian Speed	Decision Sightline				
intersection	Design Speed	Required	East	West		
Cedar Creek Rd (RR 97) & East Site Access			Yes>500m	Yes>500m		
Cedar Creek Rd (RR 97) & East Site Access	90 km/h	288 m	Yes>500m	Yes>500m		

Analysis indicates that decision sight distance requirements on Cedar Creek Road (RR 97) are met looking east and west from the site access connections. Field observations confirm that there are currently no sight distance obstructions at the proposed access points.



Cedar Creek Rd (RR 97) - Looking east



Cedar Creek Rd (RR 97) - Looking west

# 4.4 Site Circulation Analysis

To confirm the feasibility of the proposed driveway connections and internal loading operations, AutoTURN swept-path analyses were prepared for a **53-ft tractor-trailer**. Two separate turning movement exhibits were generated:

- 1. Access point maneuvers, and
- 2. Internal loading space maneuvers.

The AutoTURN diagrams provided in **Appendix E** illustrate that the design can safely accommodate the required vehicle movements.

#### **Access Points**

The swept-path analysis for the site access points demonstrates that a 53-ft truck can enter and exit the site without encroaching on opposing lanes or adjacent curb lines. The diagrams show the vehicle completing right-turn and/or left-turn movements with adequate clearance from the driveway edges, indicating that the proposed throat widths and curb radii are sufficient to support heavy-vehicle access.

#### **Internal Loading Spaces**

A separate AutoTURN assessment was completed for the selected internal loading areas. The diagrams illustrate the truck performing low-speed maneuvers—approach, alignment, reversing, and pull-out movements—into and out of the designated loading spaces. The analysis confirms that these internal movements can be executed within the available drive aisle width, with no conflicts with parking spaces, landscaping, or building faces.

# 5.0 CAPACITY ANALYSIS

The Transportation Research Board's Highway Capacity Manual (HCM) utilizes a term "level of service" (LOS) to measure how traffic operates in intersections. There are currently six levels of service ranging from A to F. Level of Service "A" represents the best conditions and Level of Service "F" represents the worst. Synchro software was used to determine the level of service for intersections in the study area. All worksheet reports from the analyses can be found in **Appendix F**.

**Table 7** shows the control delay per vehicle associated with LOS A through F for signalized and unsignalized intersections.

Table 7 Highway Capacity Manual Levels of Service and Control Delay											
Signaliz	ed Intersection	Unsignalized Intersection									
Level of Service	Control Delay per Vehicle (sec)	Level of Service	Control Delay per Vehicle (sec)								
Α	≤ 10	А	≤ 10								
В	B > 10 and ≤ 20		> 10 and ≤ 15								
С	> 20 and ≤ 35	С	> 15 and ≤ 25								
D	> 35 and ≤ 55	D	> 25 and ≤ 35								
E	> 55 and ≤ 80	Е	> 35 and ≤ 50								
F	> 80	F	> 50								

The following assumptions were made to calibrate Synchro model:

- Ideal saturation flow rate based on the recommendations included in the Regions' Transportation Impact Studies (TIS) Requirements for Capacity Analysis, Roundabouts, Signal Warrants.
- 3.5 metres as a standard width for all lanes.
- Truck percentages based on existing volumes and expected truck trip generation

# 5.1 Cedar Creek Road (RR 97) & East Site Access

Cedar Creek Road (RR 97) & East Site Access is a proposed stop-controlled T-intersection. Future Total 2029 traffic volumes warrant a dedicated left-turn lane for westbound traffic. Consequently, the analysis includes a westbound approach with a left-turn lane and a through lane, while the eastbound and northbound approaches each consist of a single lane.

Table 8 shows the intersection analysis for Future Total (2029) Conditions

Table 8 Cedar Creek Road (RR 97) & East Site Access - Intersection Analysis Summary

Intersection LOS, Delay, and Queue by Movement - 2029 Future Total										
Intersection	Approach	Movement		AM			PM			
mersection	Approach M	Approach Movement	LOS	Delay	Queue	LOS	Delay	Queue		
	ГВ	TH Free								
	EB	RT	Free			Free				
Cedar Creek Rd &	WB	LT	Α	8.8	-	Α	8.7	-		
East Site Access	VVD	TH		Free			Free			
	ND	LT		15.4	2 m	_	10.0	3 m		
	NB	RT	C	15.4	∠ m	C	18.2	3 111		

Analysis shows that this access connection will experience good levels of service with Full Build 2029 volumes. No critical movements are expected, and minimal queues are anticipated that will not interfere with operations at the adjacent west site access or the adjacent property access.

# 5.2 Cedar Creek Road (RR 97) & West Site Access

Cedar Creek Road (RR 97) & West Site Access is a proposed stop-controlled T-intersection. Future Total 2029 traffic volumes warrant a dedicated left-turn lane for westbound traffic. Consequently, the analysis includes a westbound approach with a left-turn lane and a through lane, while the eastbound and northbound approaches each consist of a single lane.

Table 9 shows the intersection analysis for Future Total (2029) Conditions

Table 9 Cedar Creek Road (RR 97) & East Site Access - Intersection Analysis Summary

Intersection LOS, Delay, and Queue by Movement - 2029 Future Total											
Intersection	Approach	n reach May consent		AM		PM					
mersection	Approach	Movement	LOS	Delay	Queue	LOS	Delay	Queue			
	EB	TH	Ε			Free					
	ED	RT	Free			Fiee					
Cedar Creek Rd &	WB	LT	Α	8.8	-	Α	8.7	-			
West Site Access	VVB	TH	Free			Free					
	ND	LT		15.4	5.4 2 m		10.0	3 m			
	NB	RT	C	15.4			18.2	o III			

Analysis shows that this access connection will experience good levels of service with Full Build 2029 volumes. No critical movements are expected, and minimal queues are anticipated that will not interfere with operations at the adjacent east site access.

# **6.0 PARKING REQUIREMENTS**

The proposal includes a 107,639-square-foot warehouse and logistics facility with 24 docking bays.

The following table summarizes the Town's Comprehensive Zoning By-law 689-83 parking requirements.

Table 10 Zoning By-law 689-83 Parking Requirements

		Zoning By-Law 689-83						
Type of Use	Size	Rate	Minimum Parking Supply Required					
Warehousing	107,639 Sq.Ft. (9,999.99) Sq.m.	1 parking space for each 1,500 square metres of the gross floor area	7 spaces					

The development will also include 33 employee parking spaces, 242 trailer parking spaces, and 39 tractor parking spaces. The facility will staff approximately 6 warehouse personnel and 12-16 office staff (22 employees in total). The proposed vehicle parking supply meets the by-law requirements.

# 7.0 TRANSPORTATION DEMAND MANAGEMENT (TDM) PLAN

Transportation Demand Management (TDM) refers to a variety of strategies to reduce congestion, minimize the number of single-occupant vehicles, encourage non-auto modes of travel, and reduce vehicle dependency to create a sustainable transportation system. TDM strategies have multiple benefits including the following:

- Reduced auto-related emissions to improve air quality;
- Decreased traffic congestion to reduce travel time;
- Increased travel options for businesses and commuters; and
- Reduced personal transportation costs and energy consumptions.

The combined benefits listed above will assist in creating a more active and livable community through improvements to overall active transportation facilities for local businesses and the surrounding community.

Typical TDM measures that could be incorporated at the site include:

- Carpool ride sharing:
- Parking supply and management strategies;
- Establishment of Transportation Management Associations (TMAs) in employment areas;
- Programs to promote flexible working hours; and,
- Application of incident management systems and Intelligent Transportation System (ITS) innovations.

# 8.0 SUMMARY AND CONCLUSIONS

This Transportation Study is in support of a proposed warehouse development located at 3027 Cedar Creek Road (RR 97) in the Town of North Dumfries, Waterloo Region.

### **Proposed Development**

The proposal includes a 107,639-square-foot warehouse and logistics facility with 24 docking bays (19 loading, 5 repair) along its southern façade. A stormwater management facility is also proposed to the immediate west. The development will also include 33 employee parking spaces, 233 trailer parking spaces, and 39 tractor parking spaces.

### **Trip Generation**

Trip generation estimates indicate that using Gross Floor Area (GFA) as an independent variable correlates with higher vehicle and truck volumes. To account for this, the study conservatively estimated 37 and 39 new two-way vehicle trips, and 2 and 3 new two-way truck trips, to the adjacent network during weekday AM and PM peak hours, respectively.

# **Turn Lanes**

It is **recommended to construct a westbound left-turn lane** at the Cedar Creek Road (RR 97) & East Site Access intersection.

It is **recommended to construct a westbound left-turn lane** at the Cedar Creek Road (RR 97) & West Site Access intersection.

# **Traffic Impacts**

Analysis shows that this access connection will experience good levels of service with Full Build 2029 volumes. No critical movements are expected, and minimal queues are anticipated that will not interfere with operations at the adjacent site accesses or the adjacent property access.

Transportation Study – Warehouse Development - 3027 Cedar Creek Road (RR 97)

# Appendix A:

**Existing Traffic Volumes** 



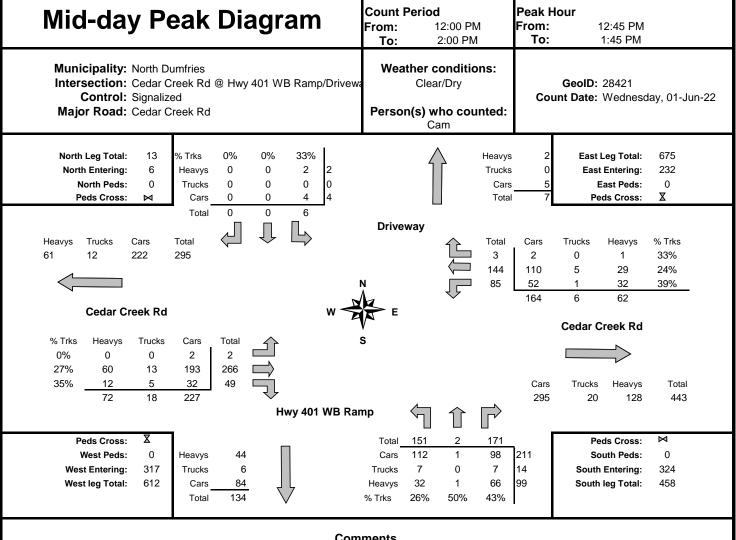
Morning Peak Diagram	Count Period         Peak Hour           From:         7:30 AM           To:         10:30 AM           To:         8:30 AM
Municipality: North Dumfries Intersection: Cedar Creek Rd @ Hwy 401 WB Ramp/Drivew Control: Signalized Major Road: Cedar Creek Rd	Weather conditions: Clear/Dry GeoID: 28421 Count Date: Wednesday, 01-Jun-22 Person(s) who counted: Cam
North Leg Total:       19       % Trks       33%       0%       100%         North Entering:       5       Heavys       0       0       2       2         North Peds:       0       Trucks       1       0       0       1         Peds Cross:       ⋈       Cars       2       0       0       2	Heavys 0   East Leg Total: 895
Total 3 0 2  Heavys Trucks Cars Total 60 15 268 343  Cedar Creek Rd W	Driveway  Total Cars Trucks Heavys % Trks  3
% Trks Heavys Trucks Cars Total 0% 0 0 2 2 16% 55 8 343 406 6% 5 0 77 60 8 422  Hwy 401 WB Rai	Cars Trucks Heavys Total 511 17 102 630
Peds Cross:         X           West Peds:         0           Heavys         35           West Entering:         490           Trucks         3           Cars         152           Total         190	Total         186         9         222           Cars         144         8         168         320           Trucks         9         1         9         19           Heavys         33         0         45         78           % Trks         23%         11%         24%    Peds Cross:  South Peds: 0  South Entering: 417  South leg Total: 607
Con	nments

To determine total vehicles entering the intersection during morning peak hour, add all leg totals entering.

Example 1: Total Entering = West leg total entering + South leg total entering + East leg total entering + North leg total entering Therefore, total vehicles entering intersection = 1,177

Example 2: Total vehicles entering from the west = eastbound left turn + eastbound through + eastbound right turn Therefore, vehicles entering from the west = 490





#### Comments

To determine total vehicles entering the intersection during mid-day peak hour, add all leg totals entering.

Total Entering = West leg total entering + South leg total entering + East leg total entering + North leg total entering Example 1: Therefore, total vehicles entering intersection = 879

Total vehicles entering from the west = eastbound left turn + eastbound through + eastbound right turn Example 2: Therefore, vehicles entering from the west = 317



Afternoon Peak Diagram	Count Period         Peak Hour           From:         3:00 PM         From:         4:15 PM           To:         6:00 PM         To:         5:15 PM										
Municipality: North Dumfries Intersection: Cedar Creek Rd @ Hwy 401 WB Ramp/Drivew Control: Signalized Major Road: Cedar Creek Rd	Weather conditions: Clear/Dry GeolD: 28421 Count Date: Wednesday, 01-Jun-22 Person(s) who counted: Cam										
North Leg Total:         25         % Trks         0%         0%         0%           North Entering:         23         Heavys         0         0         0         0           North Peds:         0         Trucks         0         0         0         0           Peds Cross:         ⋈         Cars         6         4         13         23	Heavys 0										
Total 6 4 13  Heavys Trucks Cars Total 39 7 571 617  Cedar Creek Rd W	Driveway  Total Cars Trucks Heavys % Trks  2 2 0 0 0 0%  236 220 4 12 7%  188 3 30 15%  410 7 42										
% Trks Heavys Trucks Cars Total 0% 0 0 0 0 11% 28 6 286 320 6% 4 1 73 32 7 359  Hwy 401 WB Rai	Cedar Creek Rd  Cars Trucks Heavys Total 423 12 101 536										
Peds Cross:         X           West Peds:         0           Heavys         34           West Entering:         398           Trucks         4           Cars         265           Total         303	Total         375         0         203           Cars         345         0         124         469         South Peds: 0         0           Trucks         3         0         6         9         South Entering: 578         578           Heavys         27         0         73         100         South leg Total: 881           % Trks         8%         0%         39%         South leg Total: 881										
Corr	Comments										

To determine total vehicles entering the intersection during afternoon peak hour, add all leg totals entering.

Total Entering = West leg total entering + South leg total entering + East leg total entering + North leg total entering

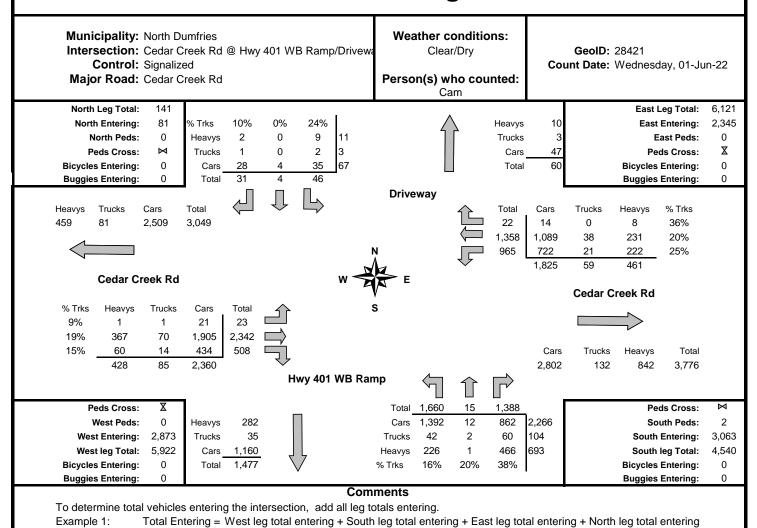
Example 1: Therefore, total vehicles entering intersection = 1,458

Example 2: Total vehicles entering from the west = eastbound left turn + eastbound through + eastbound right turn

Therefore, vehicles entering from the west = 398



# **Total Count Diagram**



Total vehicles entering from the west = eastbound left turn + eastbound through + eastbound right turn

Therefore, total vehicles entering intersection = 8,362

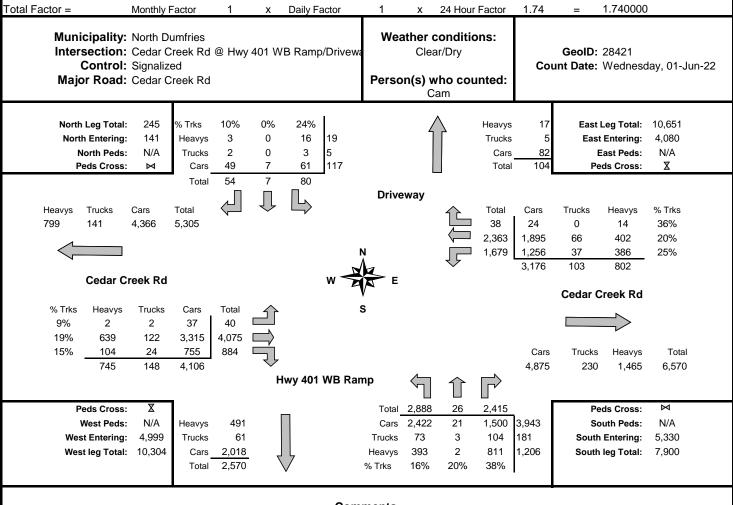
Therefore, vehicles entering from the west = 2,873

Example 2:

Notes: None



# **Estimated Daily Traffic**



#### Comments

To determine the EDT (Estimated Daily Traffic), add all four leg totals and divide by two.

This will give you the approximate number of vehicles entering and exiting the intersection in a 24-hour period

Example: (West leg total + South leg total + East leg total + North leg total) EDT = 14,550

2



# **Peak Hour Factor By Movement**

Municipality: North Dumfries

Intersection: Cedar Creek Rd @ Hwy 401 WB Ramp/Drivewa

Control: Signalized

Major Road: Cedar Creek Rd

Weather conditions:

Clear/Dry

Person(s) who counted:

Cam

GeoID: 28421

Count Date: Wednesday, 01-Jun-22

North Approach PHF

AM Peak: 0.42 Mid-day Peak: 0.50 PM Peak: 0.30

 Movement

 PM
 0.25
 0.33
 0.33

 MID
 0.00
 0.00
 0.50

 AM
 0.38
 0.00
 0.25

AM Peak: 0.95 Mid-day Peak: 0.84

PM Peak:

Peak: 0.89

East Approach PHF

AM AM Peak Hour MID Mid-day Peak H

MID Mid-day Peak Hour PM PM Peak Hour **Driveway** 

4 1 5





Cedar Creek Rd

Cedar Creek Rd

PM MID AM
0.00 0.25 0.50
0.83 0.83 0.84
0.89 0.77 0.89





West Approach PHF AM Peak: 0.85 Mid-day Peak: 0.83 PM Peak: 0.87 South Approach PHF
AM Peak: 0.85
Mid-day Peak: 0.89
PM Peak: 0.89

Comments

Intersection: Cedar Creek Rd @ Hwy 401 WB Ra

GeoID: 28421

Municipality: North Dumfries
Major Road: Cedar Creek Rd

Intersection Control: Signalized

Date: Wednesday, 01-Jun-22

Name: Cam Weather: Clear/Dry

EASTBOUND					ID	WESTBOUND			NORTHBOUND			SOUTHBOUND				
A	Approach Control		Signalized			s	Signalize	d	Signalized			Signalized				
															TOTAL	TOTAL HOUR
7:30	to	7:45	0	121	23	23	43	2	40	2	55	0	0	0	309	
7:45	to	8:00	1	115	20	33	34	0	49	4	69	2	0	1	328	
8:00	to	8:15	0	81	19	30	39	1	48	2	59	0	0	2	281	
8:15	to	8:30	1	89	20	22	38	0	49	1	39	0	0	0	259	1,177
8:30	to	8:45	0	96	16	28	46	1	28	0	30	0	0	2	247	1,115
8:45	to	9:00	3	82	16	22	42	0	25	2	37	0	0	2	231	1,018
9:00	to	9:15	1	53	12	25	48	4	24	0	38	1	0	1	207	944
9:15	to	9:30	1	57	13	26	41	1	30	0	28	1	0	1	199	884
9:30	to	9:45	2	62	16	24	29	2	23	0	40	4	0	0	202	839
9:45	to	10:00	0	56	21	15	43	0	22	0	35	0	0	1	193	801
10:00	to	10:15	0	67	14	15	33	0	28	0	21	0	0	1	179	773
10:15	to Peak	10:30	0	66	11	33	29	0	35	0	35	2	0	0	211	785
7:30	to	8:30	2	406	82	108	154	3	186	9	222	2	0	3	1,177	
# o	f truck	s in peak	0	8	0	3	5	0	9	1	9	0	0	1	36	
# of	heavie	es in peak	0	55	5	30	27	0	33	0	45	2	0	0	197	
% ł	neavie	s (Total)	0%	16%	6%	31%	21%	0%	23%	11%	24%	100%	0%	33%	20%	
		-														
12:00	to	12:15	0	60	9	16	36	0	34	0	33	4	0	7	199	
12:15	to	12:30	6	67	8	18	37	2	28	0	49	0	0	3	218	
12:30	to	12:45	4	64	16	19	47	1	29	1	35	0	0	2	218	
12:45	to	13:00	0	58	15	18	35	1	42	1	43	1	0	0	214	849
13:00	to	13:15	0	59	10	25	30	0	32	0	40	0	0	0	196	846
13:15	to	13:30	0	80	16	23	45	1	33	1	41	3	0	0	243	871
13:30	to	13:45	2	69	8	19	34	1	44	0	47	2	0	0	226	879
13:45	to	14:00	0	58	12	22	26	2	29	0	51	1	0	0	201	866
	ay Pea	k Hour														
12:45	to	13:45	2	266	49	85	144	3	151	2	171	6	0	0	879	
		s in peak	0	13	5	1	5	0	7	0	7	0	0	0	38	
		es in peak	0	60	12	32	29	1	32	1	66	2	0	0	235	
% ł	neavie	es (Total)	0%	27%	35%	39%	24%	33%	26%	50%	43%	33%	0%	0%	31%	
15:00	to	15:15	2	65	19	26	42	0	45	0	55	0	0	0	254	
15:15	to	15:30	0	48	13	31	40	1	65	0	48	2	0	0	248	
15:30	to	15:45	0	75	16	45	56	0	77	0	49	3	0	1	322	
15:45	to	16:00	0	87	15	33	47	0	92	0	49	1	0	0	324	1,148
16:00	to	16:15	0	81	20	52	48	0	100	0	59	3	0	1	364	1,258
16:15	to	16:30	0	67	17	58	59	0	110	0	52	0	0	0	363	1,373
16:30	to	16:45	0	96	19	53	67	1	106	0	54	2	0	0	398	1,449
16:45	to	17:00	0	75	20	46	45	1	91	0	48	1	1	0	328	1,453
17:00	to	17:15	0	82	22	64	65	0	68	0	49	10	3	6	369	1,458
17:15	to	17:30	0	77	19	38	55	0	89	0	31	2	0	0	311	1,406
17:30	to	17:45	0	73	21	30	45	0	86	1	38	1	0	0	295	1,303
17:45	to	18:00	0	56	12	33	34	0	59	0	31	0	0	0	225	1,200
	Peak		•	000	70	004	000	•	075	•	000	40		•	4 450	
16:15	to	17:15	0	320	78	221	236	2	375	0	203	13	4	6	1,458	
		s in peak	0	6	1	3	4	0	3	0	6	0	0	0	23	
		es in peak	0	28	4	30	12	0	27	0	73	0	0	0	174	
% ł	neavie	es (Total)	0%	11%	6%	15%	7%	0%	8%	0%	39%	0%	0%	0%	14%	

Intersection: Cedar Creek Rd @ Hwy 401 WB Ran Intersection Control: Signalized

GeolD: 28421 Date: Wednesday, 01-Jun-22

Municipality:North DumfriesName:CamMajor Road:Cedar Creek RdWeather:Clear/Dry

PEDESTRIAN CROSSING											
					Crossing	g Approach					
	Time		East App.	West App.	North App.	South App.	TOTAL	TOTAL HOUR			
7:30	to	7:45	0	0	0	0	0				
7:45	to	8:00	0	0	0	0	0				
8:00	to	8:15	0	0	0	0	0				
8:15	to	8:30	0	0	0	0	0	0			
8:30	to	8:45	0	0	0	0	0	0			
8:45	to	9:00	0	0	0	0	0	0			
9:00	to	9:15	0	0	0	0	0	0			
9:15	to	9:30	0	0	0	0	0	0			
9:30	to	9:45	0	0	0	1	1	1			
9:45	to	10:00	0	0	0	0	0	1			
10:00	to	10:15	0	0	0	0	0	1			
10:15	to	10:30	0	0	0	0	0	1			
AM I	Peak I	lour					1				
7:30	to	8:30	0	0	0	0	0				
12:00	to	12:15	0	0	0	1	1				
12:15	to	12:30	0	0	0	0	0				
12:30	to	12:45	0	0	0	0	0				
12:45	to	13:00	0	0	0	0	0	1			
13:00	to	13:15	0	0	0	0	0	0			
13:15	to	13:30	0	0	0	0	0	Ö			
13:30	to	13:45	0	0	0	0	0	ő			
13:45	to	14:00	0	0	0	0	0	0			
		k Hour	Ŭ	- U	- U	Ŭ	1	Ŭ			
12:45	to	13:45	0	0	0	0	0				
12.73	10	10.40	0	0	0	U	•				
15:00	to	15:15	0	0	0	0	0				
15:15	to	15:30	0	0	0	0	0				
15:30	to	15:45	0	0	0	0	0				
15:45	to	16:00	0	0	0	0	0	0			
16:00	to	16:00	0	0	0	0	0	0			
16:00		16:13	0	0	0	0	0	0			
16:30	to	16:30	0	0	0	0	0	0			
	to	16:45	•	-	-	_	_	_			
16:45	to		0	0	0	0	0	0			
17:00	to	17:15	0	0	0	0	0	0			
17:15	to	17:30	0	0	0	0	0	0			
17:30	to	17:45	0	0	0	0	0	0			
17:45	to	18:00	0	0	0	0	0	0			
	Peak I		0	6	6	6	0				
16:15	to	17:15	0	0	0	0	0				



Morning Peak Diagram	Count Period         Peak Hour           From:         7:30 AM           To:         10:30 AM           To:         8:30 AM										
Municipality: North Dumfries Intersection: Northumberland St @ Cedar Creek Rd Control: Signalized Major Road: Cedar Creek Rd	Weather conditions: Clear/Dry GeoID: 21462 Count Date: Wednesday, 01-Jun-22 Person(s) who counted: Cam										
North Leg Total:       329       % Trks       14%       12%       7%         North Entering:       144       Heavys       0       8       2       10         North Peds:       0       Trucks       1       3       1       5         Peds Cross:       ⋈       Cars       6       83       40       129	Heavys 7   East Leg Total: 777										
Total 7 94 43  Heavys Trucks Cars Total 37 10 241 288  Cedar Creek Rd W	Total Cars Trucks Heavys % Trks  11 9 1 1 18%  150 118 5 27 21%  150 125 5 20 17%  252 11 48  Cedar Creek Rd										
% Trks Heavys Trucks Cars Total 0% 0 0 5 5 13% 24 4 183 7% 6 0 80 30 4 268  Northumberland	Cars Trucks Heavys Total 415 7 44 466										
Peds Cross:         X           West Peds:         0           Heavys         34           West Entering:         302           Trucks         8           Cars         288           Total         330	Total       131       169       212       Peds Cross:       M         Cars       117       163       192       472       South Peds:       0         Trucks       4       0       2       6       South Entering:       512         Heavys       10       6       18       34       South leg Total:       842         % Trks       11%       4%       9%       842										
Com	Comments										

To determine total vehicles entering the intersection during morning peak hour, add all leg totals entering.

Example 1: Total Entering = West leg total entering + South leg total entering + East leg total entering + North leg total entering Therefore, total vehicles entering intersection = 1,269

Example 2: Total vehicles entering from the west = eastbound left turn + eastbound through + eastbound right turn Therefore, vehicles entering from the west = **302** 



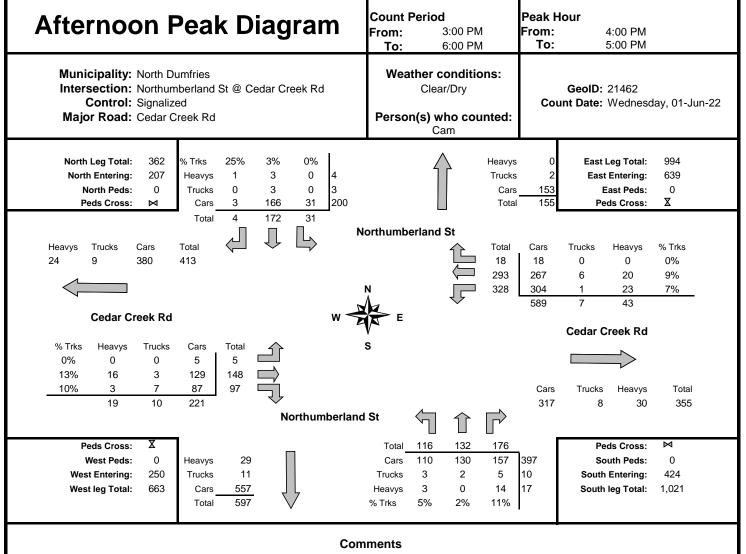
Mid-day Peak Diagram	Count Period         Peak Hour           From:         12:00 PM         From:         12:30 PM           To:         2:00 PM         To:         1:30 PM										
Municipality: North Dumfries Intersection: Northumberland St @ Cedar Creek Rd Control: Signalized Major Road: Cedar Creek Rd	Weather conditions: Clear/Dry GeolD: 21462 Count Date: Wednesday, 01-Jun-22 Person(s) who counted: Cam										
North Leg Total:       193       % Trks       20%       17%       16%         North Entering:       116       Heavys       1       9       1       11         North Peds:       0       Trucks       0       6       3       9         Peds Cross:       ⋈       Cars       4       71       21       96	Heavys 5										
Total 5 86 25  Heavys Trucks Cars Total 34 1 137 172  Cedar Creek Rd  % Trks Heavys Trucks Cars Total 4	Total Cars Trucks Heavys % Trks  10 9 0 1 10%  126 95 1 30 25%  154 121 10 23 21%  225 11 54  Cedar Creek Rd										
0% 0 0 5 5 5 30% 34 6 94 134 22% 8 3 38 49 Northumberland	Cars Trucks Heavys Total 224 17 58 299  St										
Peds Cross: X           West Peds:         0         Heavys         40           West Entering:         188         Trucks         19           West leg Total:         360         Cars         230           Total         289	Total       41       62       140       Peds Cross:       ▶         Cars       38       52       109       199       South Peds:       0         Trucks       0       6       8       14       South Entering:       243         Heavys       3       4       23       30       South leg Total:       532         % Trks       7%       16%       22%       24%       30<										
Com	Comments										

To determine total vehicles entering the intersection during mid-day peak hour, add all leg totals entering.

Example 1: Total Entering = West leg total entering + South leg total entering + East leg total entering + North leg total entering Therefore, total vehicles entering intersection = 837

Example 2: Total vehicles entering from the west = eastbound left turn + eastbound through + eastbound right turn Therefore, vehicles entering from the west = 188





To determine total vehicles entering the intersection during afternoon peak hour, add all leg totals entering.

Total Entering = West leg total entering + South leg total entering + East leg total entering + North leg total entering

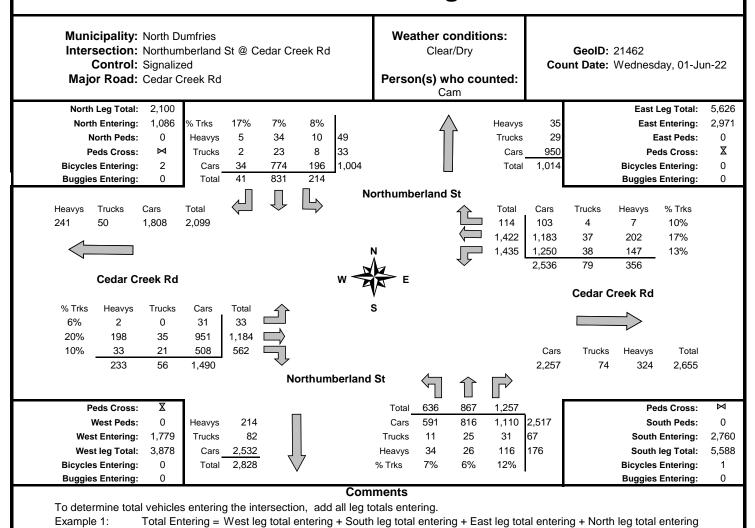
Example 1: Therefore, total vehicles entering intersection = 1,520

Example 2: Total vehicles entering from the west = eastbound left turn + eastbound through + eastbound right turn

Therefore, vehicles entering from the west = 250



# **Total Count Diagram**



Total vehicles entering from the west = eastbound left turn + eastbound through + eastbound right turn

Therefore, total vehicles entering intersection = 8,596

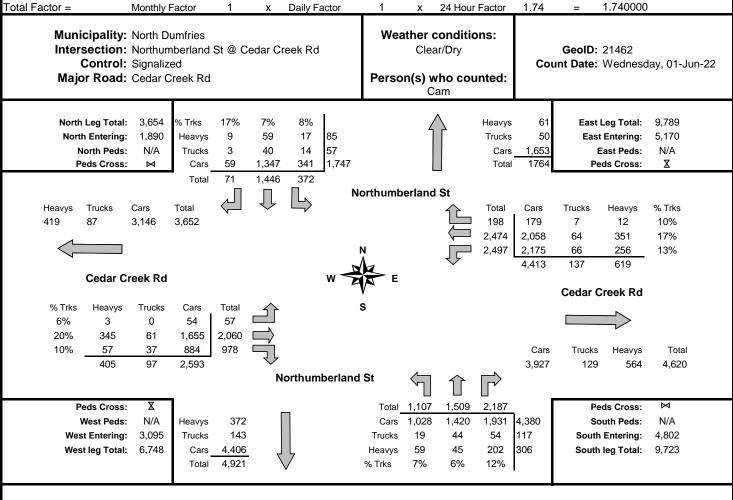
Therefore, vehicles entering from the west = 1,779

Example 2:

Notes: None



# **Estimated Daily Traffic**



#### Comments

To determine the EDT (Estimated Daily Traffic), add all four leg totals and divide by two.

This will give you the approximate number of vehicles entering and exiting the intersection in a 24-hour period

Example: (West leg total + South leg total + East leg total + North leg total) EDT = 14,957

2



# **Peak Hour Factor By Movement**

Municipality: North Dumfries

Intersection: Northumberland St @ Cedar Creek Rd

Control: Signalized

Major Road: Cedar Creek Rd

Weather conditions:

Clear/Dry

Person(s) who counted:

Cam

GeoID: 21462

Count Date: Wednesday, 01-Jun-22

AM Peak:

PM Peak:

Mid-day Peak:

East Approach PHF

0.95

0.91

0.91

North Approach PHF

AM Peak: 0.82 Mid-day Peak: 0.74 PM Peak: 0.91

Movement

PM 0.50 0.90 0.86

MID 0.63 0.74 0.69

AM 0.35 0.73 0.63

Northumberland St

AM AM Peak Hour

MID Mid-day Peak Hour

PM PM Peak Hour





AM	MID	PM	
0.55	0.63	0.75	
0.87	0.95	0.93	
0.87	0.88	0.87	

Cedar Creek Rd

Cedar Creek Rd

PM MID AM
0.63 0.63 0.42
0.86 0.70 0.85
0.84 0.72 0.72





Northumberland St

West Appro	ach PH
AM Peak:	0.82
/lid-day Peak:	0.71
PM Peak:	0.87

0.62 0.77 0.68 AM 0.64 0.78 0.95 MID 0.91 0.87 0.90 PM Movement South Approach PHF
AM Peak: 0.69
Mid-day Peak: 0.91
PM Peak: 0.93

Comments

Intersection: Northumberland St @ Cedar Creek

GeoID: 21462

Municipality: North Dumfries
Major Road: Cedar Creek Rd

Intersection Control: Signalized

Date: Wednesday, 01-Jun-22

Name: Cam Weather: Clear/Dry

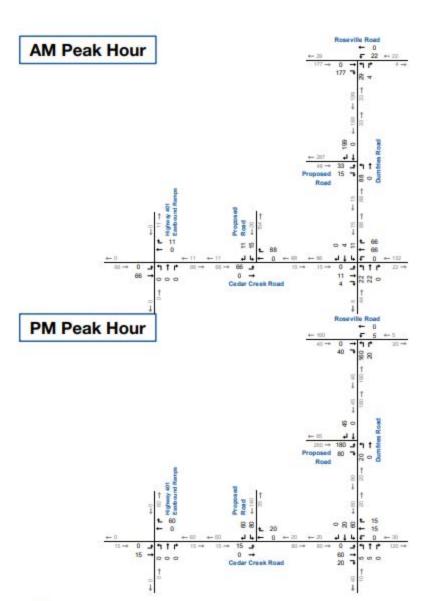
Γ		EASTBOUND			WESTBOUND			NORTHBOUND			SOUTHBOUND					
A	Approad	ch Control		Signalized	d	s	Signalize	d	s	Signalized		s	ignalized	1		
															TOTAL	TOTAL HOUR
7:30	to	7:45	3	61	13	41	37	0	53	55	78	17	22	5	385	
7:45	to	8:00	0	62	30	43	30	3	25	35	46	10	32	2	318	
8:00	to	8:15	1	43	28	34	43	5	24	42	46	9	14	0	289	4.000
8:15	to	8:30	1	45 40	15 20	32 31	40 39	3	29 32	37	42 58	7	26	0	277	1,269
8:30 8:45	to to	8:45 9:00	1 1	48 44	23	28	39 36	3 5	32 13	30 33	38	6 8	23 19	2 1	293 249	1,177 1,108
9:00	to	9:00 9:15	2	30	19	24	34	2	16	17	28	3	19	0	194	1,013
9:15	to	9:30	1	25	10	22	37	4	9	23	36	6	9	0	182	918
9:30	to	9:45	0	31	13	18	31	0	19	28	38	7	13	1	199	824
9:45	to	10:00	0	32	12	31	30	0	16	25	35	10	17	0	208	783
10:00	to	10:15	1	32	9	28	27	4	8	18	30	4	9	1	171	760
10:15	to	10:30	1	28	14	18	35	2	9	21	40	9	18	0	195	773
AM	Peak	Hour														
7:30	to	8:30	5	211	86	150	150	11	131	169	212	43	94	7	1,269	
		s in peak	0	4	0	5	5	1	4	0	2	1	3	1	26	
-		es in peak	0	24	6	20	27	1	10	6	18	2	8	0	122	
% r	neavie	es (Total)	0%	13%	7%	17%	21%	18%	11%	4%	9%	7%	12%	14%	12%	
12.00	+o	12:15		20	10	52	20	1	17	21	20	1	22	2	24.4	
12:00 12:15	to to	12:15	0 0	29 29	15	52 37	20 27	1 3	10	20	39 36	1 5	22 9	2 1	214 192	
12:13	to	12:30	1	29 48	17	36	31	3 4	9	12	35	4	9 17	1	215	
12:30	to	13:00	1	18	11	44	33	3	11	17	37	9	29	1	213	835
13:00	to	13:15	2	28	11	32	29	1	16	20	31	8	19	2	199	820
13:15	to	13:30	1	40	10	42	33	2	5	13	37	4	21	1	209	837
13:30	to	13:45	2	32	8	41	33	1	9	20	38	4	17	3	208	830
13:45	to	14:00	0	29	6	22	25	4	17	23	31	6	20	2	185	801
Midda	ay Pea	ık Hour														
12:30	to	13:30	5	134	49	154	126	10	41	62	140	25	86	5	837	
		s in peak	0	6	3	10	1	0	0	6	8	3	6	0	43	
		es in peak	0	34	8	23	30	1	3	4	23	1	9	1	137	
% ł	neavie	es (Total)	0%	30%	22%	21%	25%	10%	7%	16%	22%	16%	17%	20%	22%	
15:00	to	15:15	1	41	18	40	34	5	12	19	38	5	28	1	242	
15:15	to to	15:30	1	21	14	43	5 <del>1</del>	3	17	24	23	9	33	1	242	
15:30	to	15:45	1	60	25	61	70	8	22	25	28	4	37	3	344	
15:45	to	16:00	1	41	22	57	70	6	19	39	43	5	47	2	352	1,178
16:00	to	16:15	2	43	27	76	74	4	27	38	49	7	37	0	384	1,320
16:15	to	16:30	2	32	17	84	79	4	31	23	42	7	43	1	365	1,445
16:30	to	16:45	0	43	29	94	77	4	26	34	48	9	44	2	410	1,511
16:45	to	17:00	1	30	24	74	63	6	32	37	37	8	48	1	361	1,520
17:00	to	17:15	1	39	22	64	81	8	33	32	48	5	31	1	365	1,501
17:15	to	17:30	2	38	15	71	75	8	27	30	44	5	58	3	376	1,512
17:30	to	17:45	1	33	35	65	61	6	21	36	37	6	24	0	325	1,427
17:45	to	18:00	1	29	20	50	37	2	22	20	21	7	26	1	236	1,302
16:00	Peak to	Hour 17:00	5	148	97	328	293	18	116	132	176	31	172	4	1,520	
		s in peak	0	3	7	1	6	0	3	2	5	0	3	0	30	
		s in peak	0	3 16	3	23	20	0	3	0	14	0	3	1	83	
		s (Total)	0%	13%	10%	7%	9%	0%	5%	2%	11%	0%	3%	25%	7%	
/0 1	a v i c	J ( i Jiai)	J /U	10/0	10/0	1 /0	J /0	J /U	J /0	<b>-</b> /0	11/0	J /0	J /0	20/0	1 /0	

Intersection: Northumberland St @ Cedar Creek R Intersection Control: Signalized

GeoID: 21462 Date: Wednesday, 01-Jun-22

Municipality:North DumfriesName:CamMajor Road:Cedar Creek RdWeather:Clear/Dry

				PEDEST	RIAN CROSSI	NG		
					Crossin	g Approach		
	Time		East App.	West App.	North App.	South App.	TOTAL	TOTAL HOUR
7:30	to	7:45	0	0	0	0	0	
7:45	to	8:00	0	0	0	0	0	
8:00	to	8:15	0	0	0	0	0	
8:15	to	8:30	0	0	0	0	0	0
8:30	to	8:45	0	0	0	0	0	0
8:45	to	9:00	0	0	0	0	0	0
9:00	to	9:15	0	0	0	0	0	0
9:15	to	9:30	0	0	0	0	0	0
9:30	to	9:45	0	0	0	0	0	0
9:45	to	10:00	0	0	0	0	0	0
10:00	to	10:15 10:30	0 0	0	0 0	0 0	0 0	0
10:15	to Peak I		U	0	0	U	0	U
7:30	to	8:30	0	0	0	0	0	
1.30	ιο	0.30	U	0	U	U	0	
12:00	to	12:15	0	0	0	0	0	
12:15	to	12:30	0	0	Ö	Ö	0	
12:30	to	12:45	0	0	Ö	0	0	
12:45	to	13:00	0	0	0	0	0	0
13:00	to	13:15	0	0	0	0	0	0
13:15	to	13:30	0	0	0	0	0	0
13:30	to	13:45	0	0	0	0	0	0
13:45	to	14:00	0	0	0	0	0	0
Midda	y Pea	k Hour					0	
12:30	to	13:30	0	0	0	0	0	
15:00	to	15:15	0	0	0	0	0	
15:15	to	15:30	0	0	0	0	0	
15:30	to	15:45	0	0	0	0	0	_
15:45	to	16:00	0	0	0	0	0	0
16:00	to	16:15	0	0	0	0	0	0
16:15	to	16:30	0	0	0	0	0	0
16:30	to	16:45	0	0	0	0	0	0
16:45	to	17:00	0	0	0	0	0	0
17:00	to	17:15	0	0	0	0	0	0
17:15 17:30	to	17:30	0	0	0	0	0	0
17:30	to to	17:45 18:00	0 0	0 0	0 0	0 0	0 0	0
	Peak I		U	U	U	U	0	0
16:00	to	17:00	0	0	0	0	0	
10.00	ı	17.00	J	U	U	U	J	

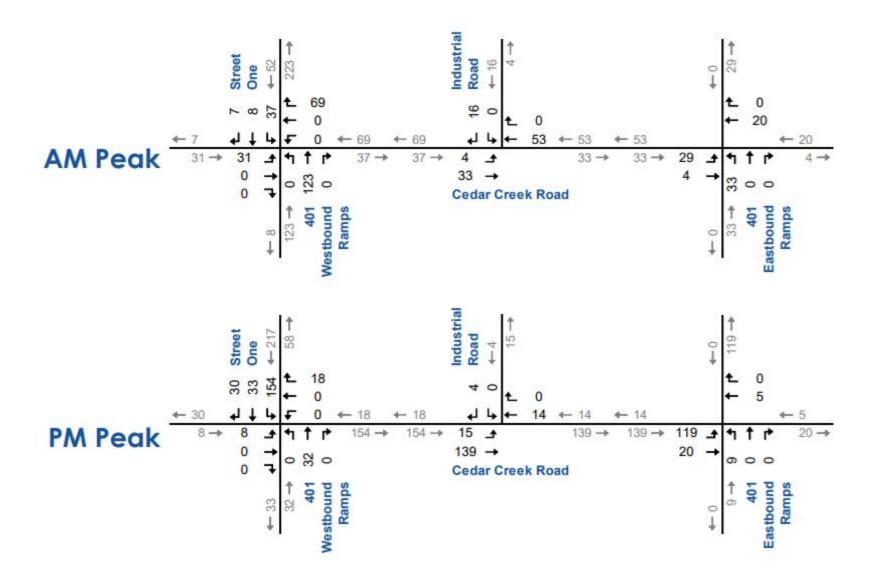




**Site-Generated Traffic Forecasts** 

Regional Roads 97 at 47 Industrial Lands 200319

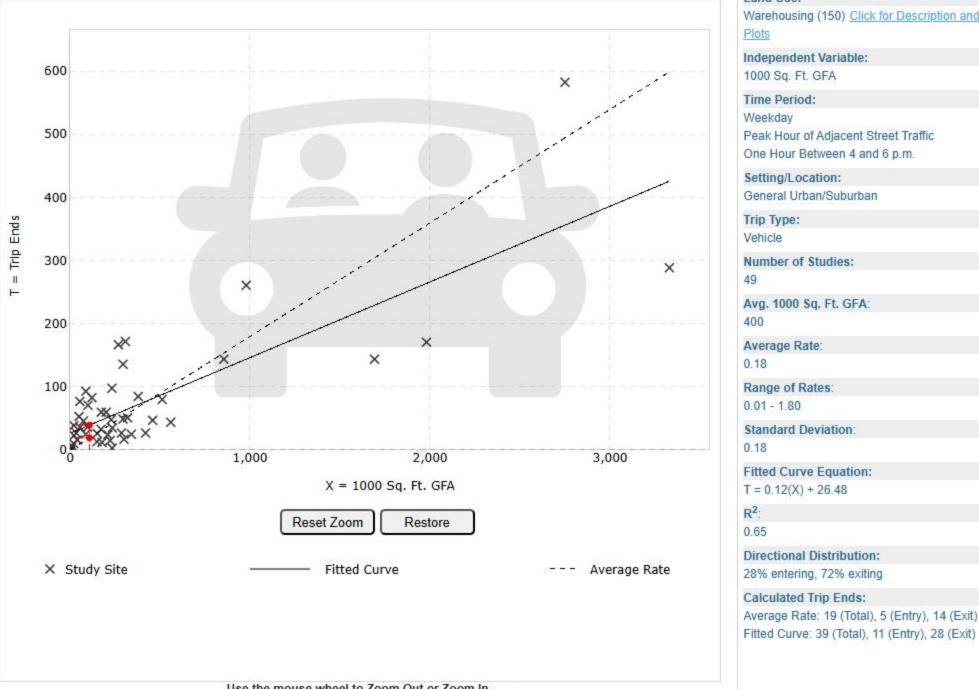






**Site-Generated Traffic Forecasts** 

DATA SOURCE:	
Trip Generation Manual, 11th Ed	~
SEARCH BY LAND USE CODE:	
150	
LAND USE GROUP:	
(100-199) Industrial	~
LAND USE:	
150 - Warehousing	~
LAND USE SUBCATEGORY:	
All Sites	~
SETTING/LOCATION:	
General Urban/Suburban	~
INDEPENDENT VARIABLE (IV):	
1000 Sq. Ft. GFA	~
TIME PERIOD:	
Weekday, Peak Hour of Adjacent Street Tr	affic 🕶
TRIP TYPE:	
Vehicle	~



DATA STATISTICS

Warehousing (150) Click for Description and Data

Land Use:

Independent Variable:

Peak Hour of Adjacent Street Traffic One Hour Between 4 and 6 p.m.

1000 Sq. Ft. GFA

Setting/Location: General Urban/Suburban

Number of Studies:

Avg. 1000 Sq. Ft. GFA:

Trip Type: Vehicle

400

0.18

0.18

R<sup>2</sup>:

Average Rate:

Range of Rates: 0.01 - 1.80

Standard Deviation:

**Fitted Curve Equation:** 

**Directional Distribution:** 

28% entering, 72% exiting Calculated Trip Ends:

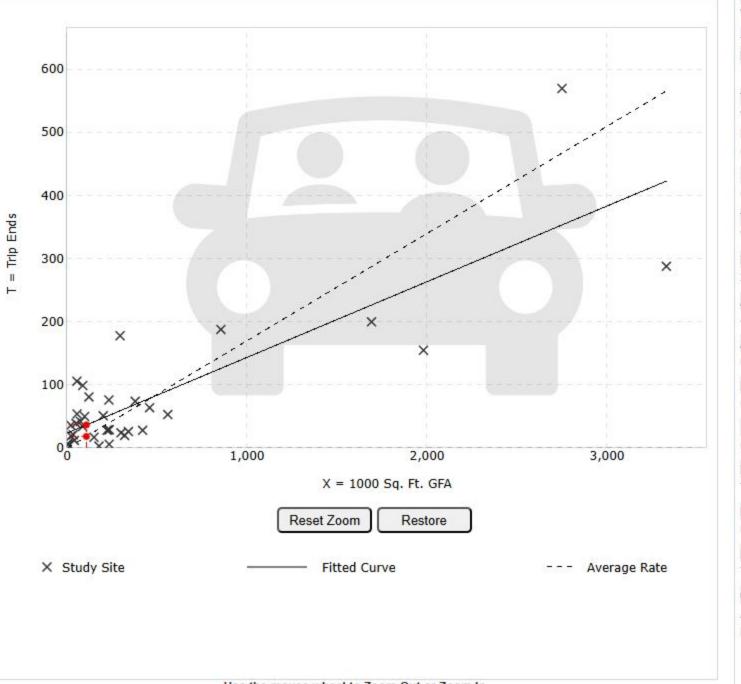
Average Rate: 19 (Total), 5 (Entry), 14 (Exit)

Time Period: Weekday

Plots

Use the mouse wheel to Zoom Out or Zoom In. Hover the mouse pointer on data points to view X and T values.

Query Filter	
DATA SOURCE:	
Trip Generation Manual, 11th Ed	~
SEARCH BY LAND USE CODE:	
150	
LAND USE GROUP:	
(100-199) Industrial	~
LAND USE:	
150 - Warehousing	~
LAND USE SUBCATEGORY:	
All Sites	~
SETTING/LOCATION:	
General Urban/Suburban	~
INDEPENDENT VARIABLE (IV):	
1000 Sq. Ft. GFA	~
TIME PERIOD:	
Weekday, Peak Hour of Adjacent Street Traffic	~
TRIP TYPE:	
Vehicle	~
ENTER IV VALUE TO CALCULATE TRIPS:	
108 Calculate	



Use the mouse wheel to Zoom Out or Zoom In.

Hover the mouse pointer on data points to view X and T values.

#### DATA STATISTICS

Land Use:

Warehousing (150) Click for Description and Data
Plots

Independent Variable:

1000 Sq. Ft. GFA

Time Period:

Weekday

Peak Hour of Adjacent Street Traffic One Hour Between 7 and 9 a.m.

Setting/Location:

General Urban/Suburban

Trip Type:

Vehicle

Number of Studies:

36

Avg. 1000 Sq. Ft. GFA:

448

Average Rate:

0.17

Range of Rates:

0.02 - 1.93

Standard Deviation:

0.19

Fitted Curve Equation:

T = 0.12(X) + 23.62

R2:

0.69

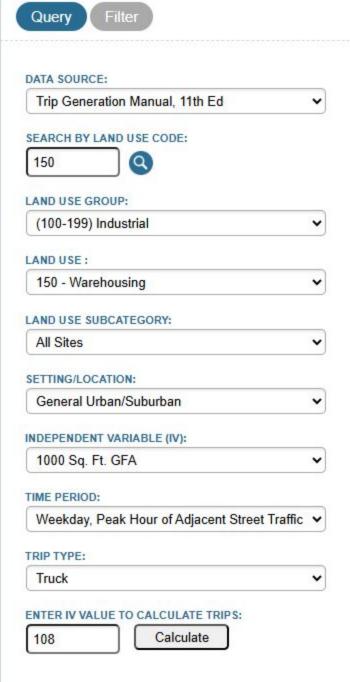
**Directional Distribution:** 

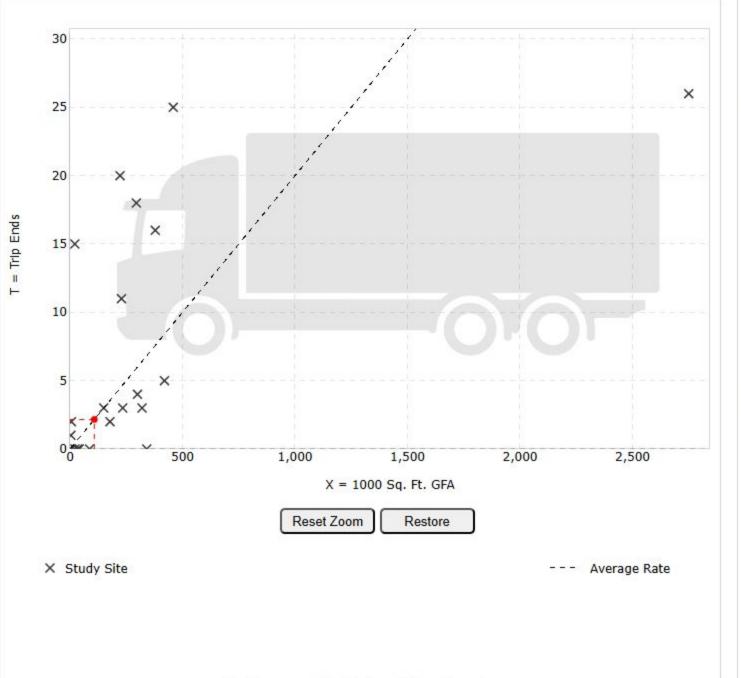
77% entering, 23% exiting

Calculated Trip Ends:

Average Rate: 18 (Total), 14 (Entry), 4 (Exit)

Fitted Curve: 37 (Total), 28 (Entry), 9 (Exit)





DATA STATISTICS

Land Use:

Warehousing (150) Click for Description and Data

Independent Variable:

1000 Sq. Ft. GFA

Time Period:

Weekday

Peak Hour of Adjacent Street Traffic

One Hour Between 7 and 9 a.m.

Setting/Location:

General Urban/Suburban

Trip Type:

Truck

**Number of Studies:** 

21

Avg. 1000 Sq. Ft. GFA:

309

Average Rate:

0.02

Range of Rates:

0.00 - 0.69

Standard Deviation:

0.05

Fitted Curve Equation:

Not Given

R<sup>2</sup>:

\*\*\*\*

**Directional Distribution:** 

52% entering, 48% exiting

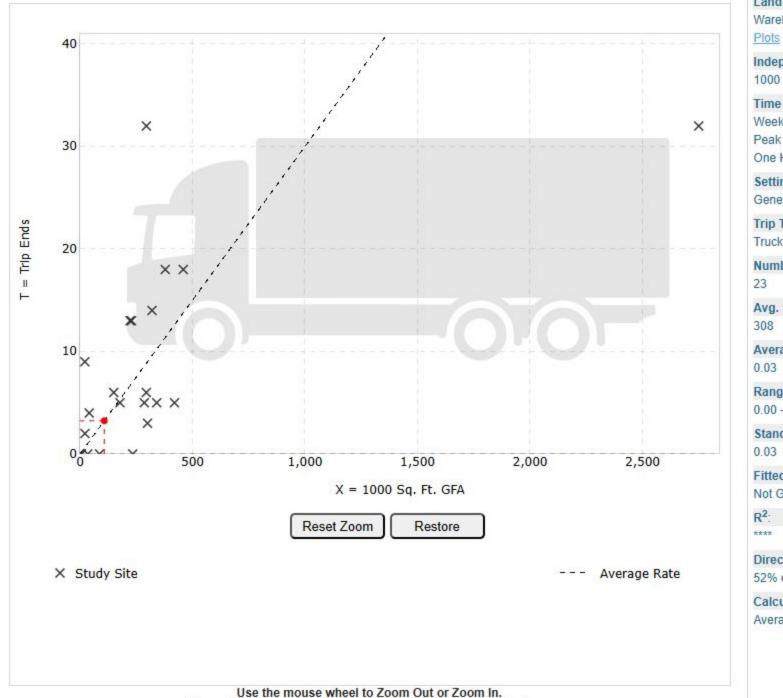
Calculated Trip Ends:

Average Rate: 2 (Total), 1 (Entry), 1 (Exit)

Use the mouse wheel to Zoom Out or Zoom In.

Hover the mouse pointer on data points to view X and T values.

Query Filter	
DATA SOURCE:	
Trip Generation Manual, 11th Ed	~
SEARCH BY LAND USE CODE:	
150	
The state of the s	
LAND USE GROUP:	
(100-199) Industrial	~
LAND USE:	
150 - Warehousing	~
LAND USE SUBCATEGORY:	
All Sites	
All Sites	
SETTING/LOCATION:	
General Urban/Suburban	~
INDEPENDENT VARIABLE (IV):	
1000 Sq. Ft. GFA	~
TIME PERIOD:	
Weekday, Peak Hour of Adjacent Street Traffic	~
rrockady, r can rrod or tojacom cacer rame	
TRIP TYPE:	
Truck	~
ENTER IV VALUE TO CALCULATE TRIPS:	
108 Calculate	



Hover the mouse pointer on data points to view X and T values.

DATA STATISTICS

Land Use:

Warehousing (150) Click for Description and Data Plots

Independent Variable:

1000 Sq. Ft. GFA

Time Period:

Weekday

Peak Hour of Adjacent Street Traffic

One Hour Between 4 and 6 p.m.

Setting/Location:

General Urban/Suburban

Trip Type:

Truck

Number of Studies:

23

Avg. 1000 Sq. Ft. GFA:

Average Rate:

0.03

Range of Rates:

0.00 - 0.42

Standard Deviation:

**Fitted Curve Equation:** 

Not Given

**Directional Distribution:** 

52% entering, 48% exiting

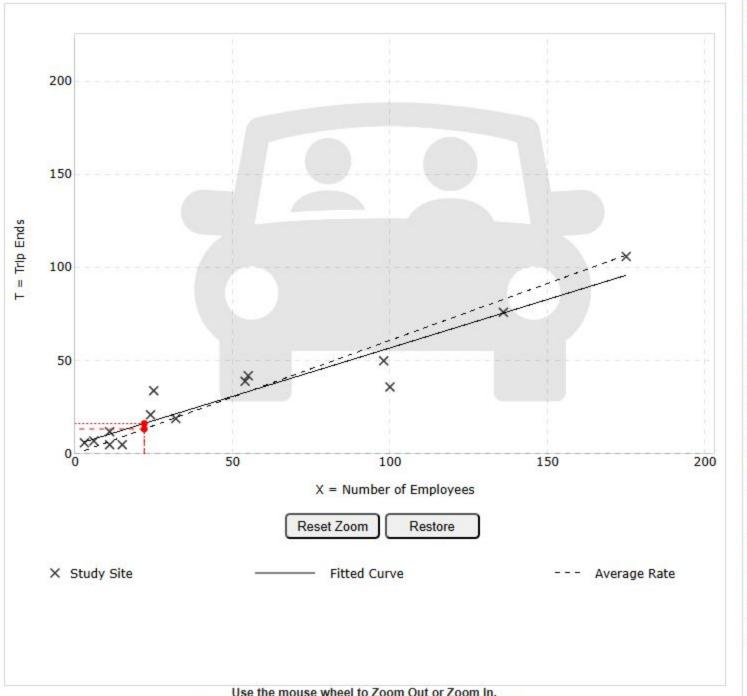
Calculated Trip Ends:

Average Rate: 3 (Total), 2 (Entry), 1 (Exit)

Trip Generation Manual, 11th Ed	~
SEARCH BY LAND USE CODE:	
150	
AND USE GROUP:	
(100-199) Industrial	~
AND USE:	
150 - Warehousing	~
AND USE SUBCATEGORY:	
All Sites	
All Oiles	- 1
SETTING/LOCATION:	:
General Urban/Suburban	~
NDEPENDENT VARIABLE (IV):	
Employees	~
IME PERIOD:	
Weekday, Peak Hour of Adjacent Street Tra	ffic V
Weekday, Feak Flour of Adjacent Street Ha	ilic v
RIP TYPE:	
Vehicle	~

Query

## Data Plot and Equation



Use the mouse wheel to Zoom Out or Zoom In.

Hover the mouse pointer on data points to view X and T values.

#### DATA STATISTICS

Land Use:

Warehousing (150) Click for Description and Data Plots

Independent Variable:

Employees

Time Period:

Weekday

Peak Hour of Adjacent Street Traffic

One Hour Between 7 and 9 a.m.

Setting/Location:

General Urban/Suburban

Trip Type:

Vehicle

Number of Studies:

4

Avg. Num. of Employees:

53

Average Rate:

0.61

Range of Rates:

0.33 - 2.00

Standard Deviation:

0.23

Fitted Curve Equation:

T = 0.52(X) + 4.93

R2:

0.91

**Directional Distribution:** 

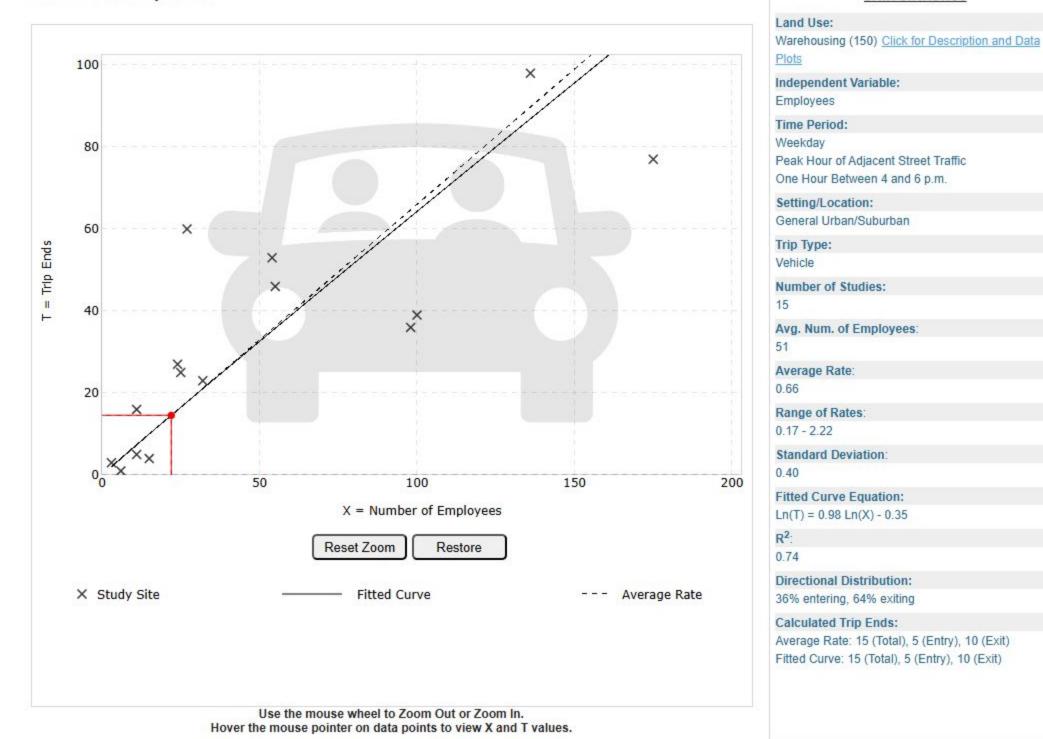
72% entering, 28% exiting

Calculated Trip Ends:

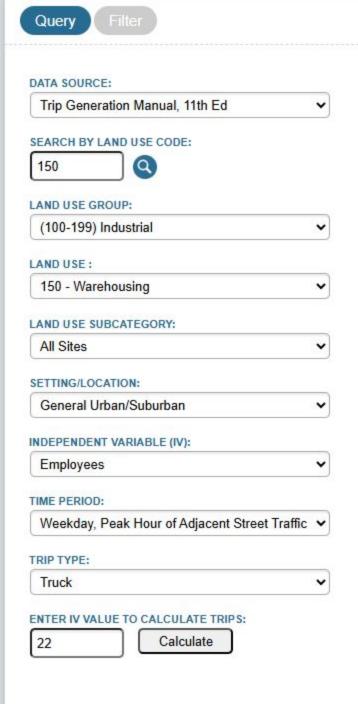
Average Rate: 13 (Total), 10 (Entry), 3 (Exit)

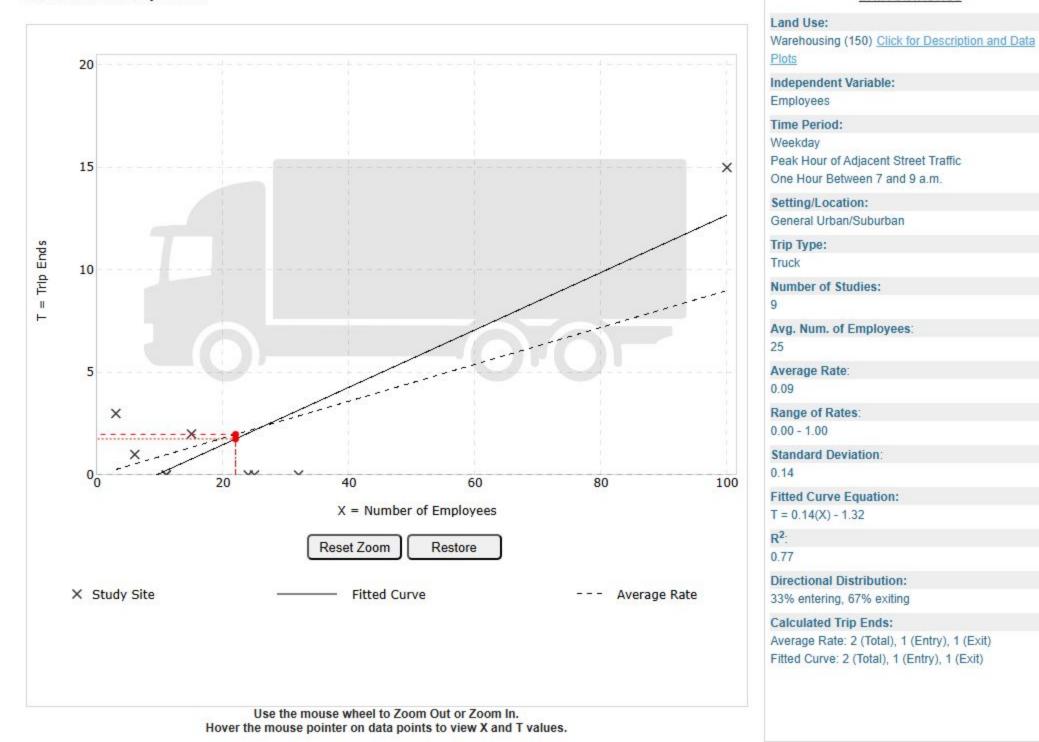
Fitted Curve: 16 (Total), 12 (Entry), 4 (Exit)

Trip Generation	Manual, 11th Ed	~
SEARCH BY LAND	USE CODE:	
150	<b>Q</b>	
LAND USE GROUP		
(100-199) Indus	strial	~
LAND USE:		
150 - Warehous	sing	~
LAND USE SUBCAT	regory:	
All Sites		~
SETTING/LOCATIO	N:	
General Urban/	Suburban	~
INDEPENDENT VAR	RIABLE (IV):	
Employees		~
TIME PERIOD:		
Weekday, Peak	Hour of Adjacent Street Traffic	~
TRIP TYPE:		
Vehicle		_

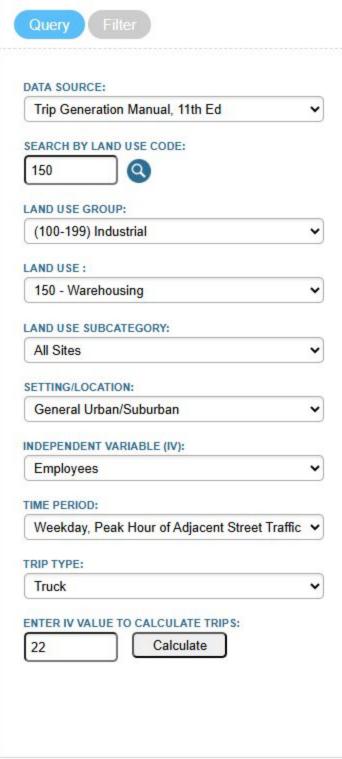


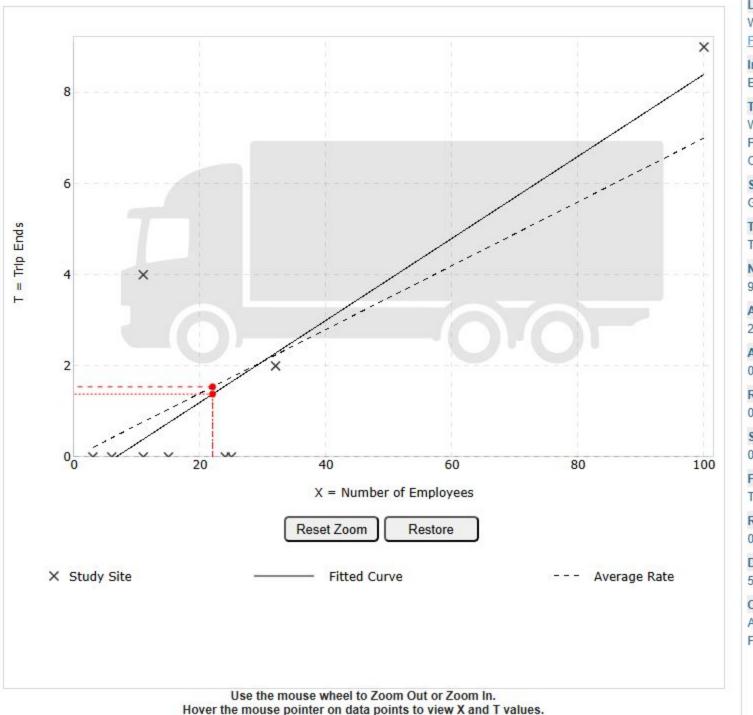
DATA STATISTICS





DATA STATISTICS





DATA STATISTICS

Land Use:

Warehousing (150) Click for Description and Data
Plots

Independent Variable:

Employees

Time Period:

Weekday

Peak Hour of Adjacent Street Traffic One Hour Between 4 and 6 p.m.

ono modi bottoon rana o p.i

Setting/Location:

General Urban/Suburban

Trip Type:

Truck

**Number of Studies:** 

9

Avg. Num. of Employees:

25

Average Rate:

0.07

Range of Rates:

0.00 - 0.36

Standard Deviation:

0.08

**Fitted Curve Equation:** 

T = 0.09(X) - 0.60

R2:

0.74

**Directional Distribution:** 

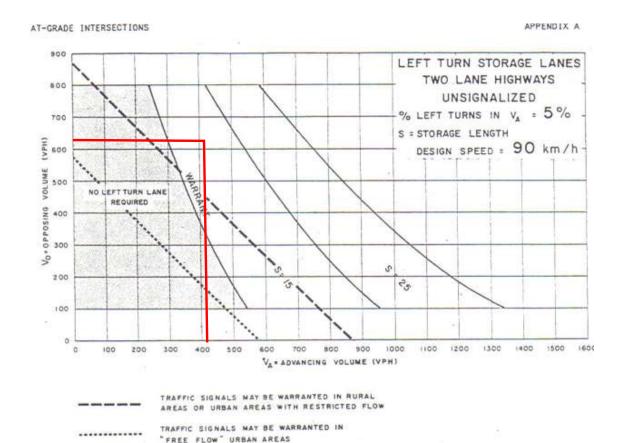
53% entering, 47% exiting

Calculated Trip Ends:

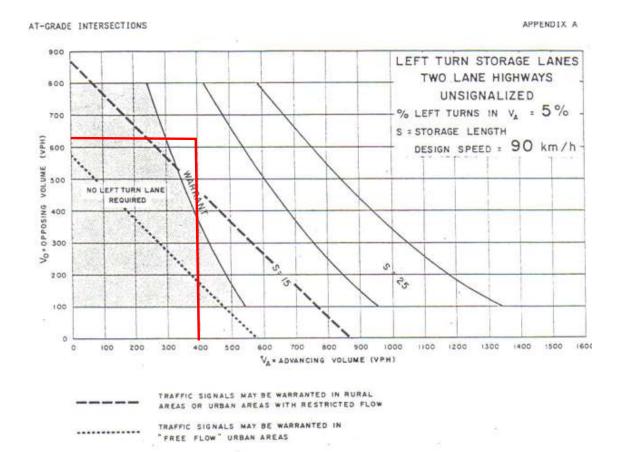
Average Rate: 2 (Total), 1 (Entry), 1 (Exit) Fitted Curve: 1 (Total), 1 (Entry), 0 (Exit)

Transportation Study – Warehouse Development - 3027 Cedar Creek Road (RR 97)
Appendix D:
Left-turn Lane Warrants/Graphs

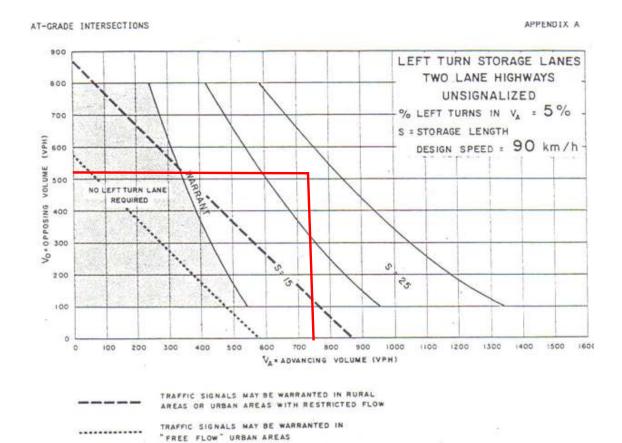
## Cedar Creek Rd at East Site Access – AM Peak Hour



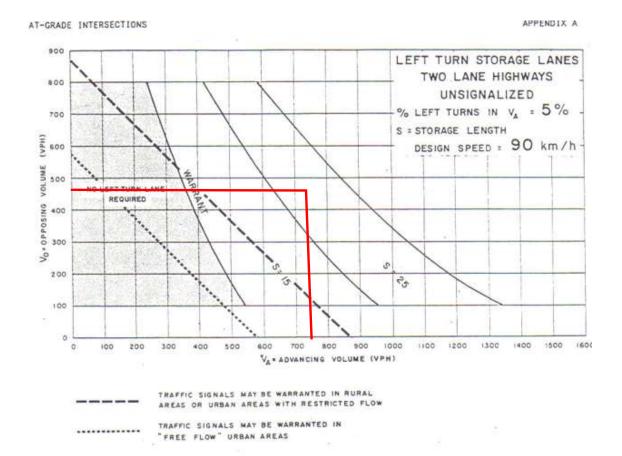
## Cedar Creek Rd at West Site Access – AM Peak Hour



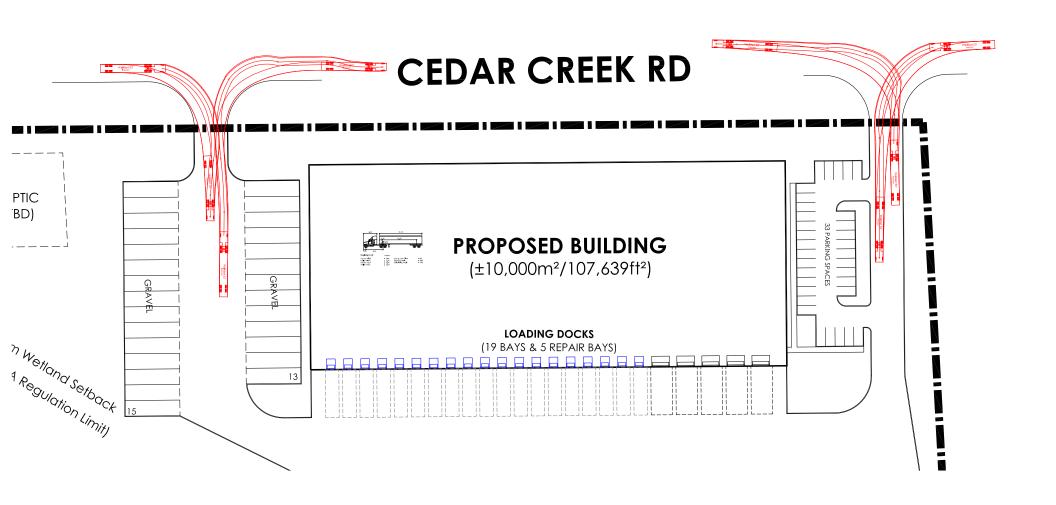
## Cedar Creek Rd at East Site Access – PM Peak Hour

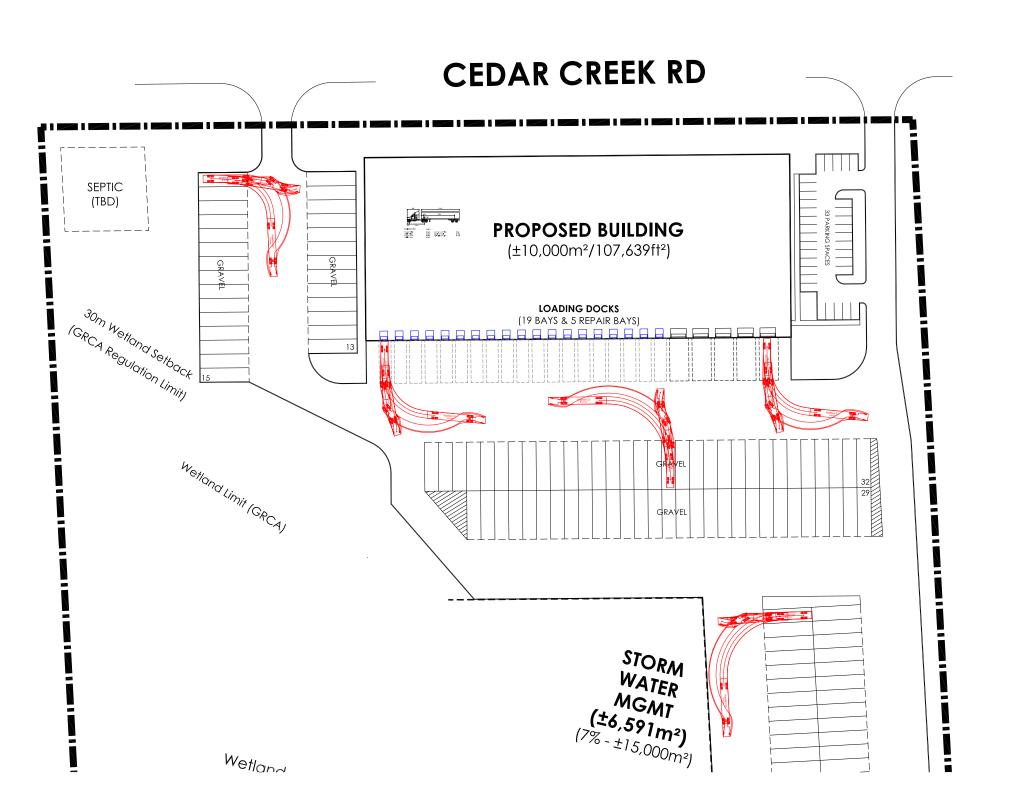


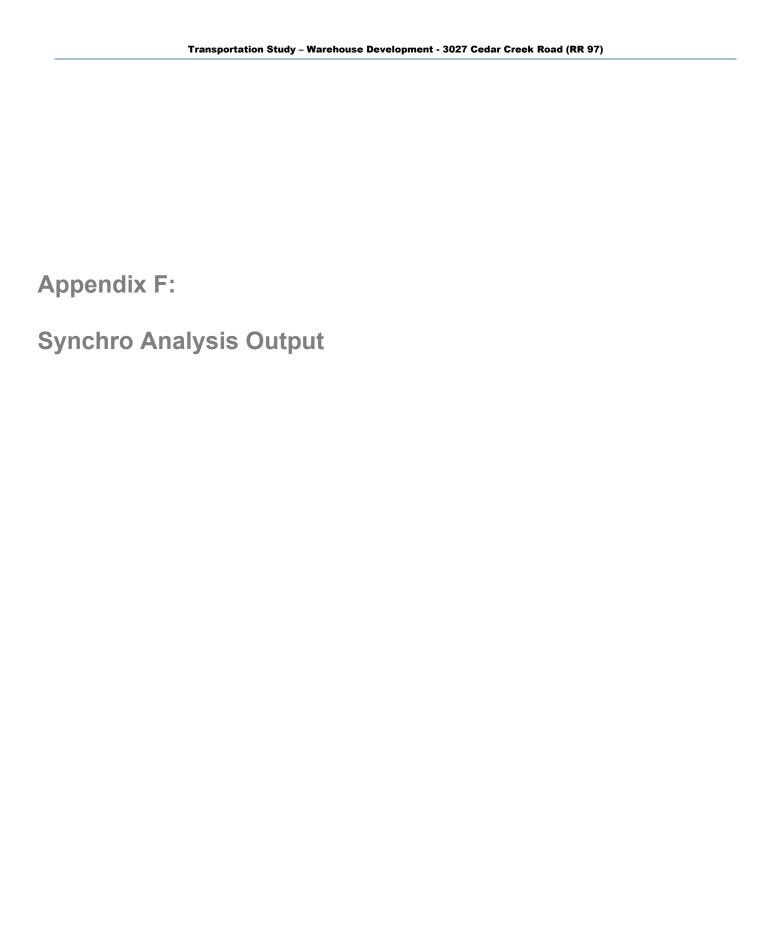
## Cedar Creek Rd at West Site Access – PM Peak Hour



Transportation Study – Warehouse Development - 3027 Cedar Creek Road (RR 97)
Appendix E:
Appendix L.
AutoTurn Analysis







Intersection						
Int Delay, s/veh	0.2					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>1</b>	LDIN	ሻ	<u>₩</u>	Y	אפא
Traffic Vol, veh/h	631	10	7	395	2	4
Future Vol, veh/h	631	10	7	395	2	4
Conflicting Peds, #/hr	0	0	0	000	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	_	-	25	-	0	-
Veh in Median Storage,	# 0	_	-	0	0	_
Grade, %	0	_	_	0	0	_
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	13	0	0	21	0	25
Mvmt Flow	631	10	7	395	2	4
IVIVIII( I IOW	001	10	- 1	333		7
	ajor1	N	Major2		Minor1	
Conflicting Flow All	0	0	641	0	1045	636
Stage 1	-	-	-	-	636	-
Stage 2	-	-	-	-	409	-
Critical Hdwy	-	-	4.1	-	6.4	6.45
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy	-	-	2.2	-	3.5	3.525
Pot Cap-1 Maneuver	-	-	953	-	256	439
Stage 1	-	-	-	-	531	-
Stage 2	-	-	-	-	675	-
Platoon blocked, %	_	_		_		
Mov Cap-1 Maneuver	-	_	953	_	254	439
Mov Cap-2 Maneuver	_	_	-	_	254	-
Stage 1	_	_	_	_	531	_
Stage 2	_	_	_	_	670	<u>-</u>
Stage 2					070	
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.2		15.4	
HCM LOS					С	
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	<u> </u>	353	-	-	953	-
Cabacity (VEII/II)					0.007	_
		0.017			0.007	-
HCM Lane V/C Ratio		0.017	-			
HCM Lane V/C Ratio HCM Control Delay (s)		15.4	-	-	8.8	-
HCM Lane V/C Ratio						-

Intersection						
Int Delay, s/veh	0.1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations		LDK	VVDL	VVD1	NDL W	אטוז
Traffic Vol, veh/h	639	8	4	<b>T</b> 396	<b>T</b>	2
Future Vol, veh/h	639	8	4	396	2	2
-	039	0	0	390	0	0
Conflicting Peds, #/hr	Free	Free	Free	Free	Stop	
Sign Control RT Channelized	-	None	riee -		Stop -	Stop None
Storage Length	-	NOHE -	25	None -	0	None
		-	25	0	0	
Veh in Median Storage	0			0	0	
Grade, %		100	100			100
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	13	13	0	21	0	0
Mvmt Flow	639	8	4	396	2	2
Major/Minor I	Major1	N	Major2	N	Minor1	
Conflicting Flow All	0	0	647	0	1047	643
Stage 1	-	-	-	-	643	-
Stage 2	_	_	_	_	404	_
Critical Hdwy	-	_	4.1	_	6.4	6.2
Critical Hdwy Stg 1	_	_	- ''	_	5.4	-
Critical Hdwy Stg 2	_	_	_	_	5.4	_
Follow-up Hdwy	_	_	2.2	_	3.5	3.3
Pot Cap-1 Maneuver	_	_	948	_	255	477
Stage 1	_	_	370	<u> </u>	527	711
Stage 2	-	-			679	
Platoon blocked, %	-	-	-	-	019	-
· · · · · · · · · · · · · · · · · · ·		-	948		251	477
Mov Cap-1 Maneuver	-	-	948	-	254	4//
Mov Cap-2 Maneuver	-	-	-	-	254	-
Stage 1	-	-	-	-	527	-
Stage 2	-	-	-	-	676	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.1		16	
HCM LOS	J		0.1		C	
					J	
Minor Lane/Major Mvm	nt 1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		331	-	-	948	-
HCM Lane V/C Ratio		0.012	-	-	0.004	-
HCM Control Delay (s)		16	-	-	8.8	-
HCM Lane LOS		С	-	-	Α	-
HCM 95th %tile Q(veh)		0	-	-	0	-

Intersection						
Int Delay, s/veh	0.3					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
		LDK				אטוו
Lane Configurations	<b>F</b> 09	2	<u>ኝ</u>	726	Y	0
Traffic Vol, veh/h	508	3	5	736	8	9
Future Vol, veh/h	508	3	5	736	8	9
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	25	-	0	-
Veh in Median Storage,	, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	9	0	20	7	0	11
Mymt Flow	508	3	5	736	8	9
WWITHER	300	3	J	700	U	3
Major/Minor N	Major1	N	Major2	N	Minor1	
Conflicting Flow All	0	0	511	0	1256	510
Stage 1	-	-	_	_	510	-
Stage 2	_	_	_	_	746	-
Critical Hdwy	_	_	4.3	_	6.4	6.31
Critical Hdwy Stg 1	_	_	-	_	5.4	-
Critical Hdwy Stg 2			_	_	5.4	_
	-	-				
Follow-up Hdwy	-	-	2.38	-		3.399
Pot Cap-1 Maneuver	-	-	968	-	191	546
Stage 1	-	-	-	-	607	-
Stage 2	-	-	-	-	472	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	968	-	190	546
Mov Cap-2 Maneuver	-	-	-	-	190	-
Stage 1	-	-	_	-	607	-
Stage 2	_	_	_	_	470	_
Olago Z					170	
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.1		18.2	
HCM LOS					С	
Minor Lane/Major Mvm	t l	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		290	-	-	968	-
HCM Lane V/C Ratio		0.059	-	-	0.005	-
HCM Control Delay (s)		18.2	-	-	8.7	-
		С		_	Α	-
HCM Lane LOS		C	-	-	/\	
HCM Lane LOS HCM 95th %tile Q(veh)		0.2	-	-	0	-

Intersection						
Int Delay, s/veh	0.2					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1>		*	<b>↑</b>	¥	
Traffic Vol, veh/h	475	3	2	742	6	6
Future Vol, veh/h	475	3	2	742	6	6
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	_	-	25	-	0	-
Veh in Median Storage	, # 0	-	-	0	0	-
Grade, %	0	_	_	0	0	_
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	9	33	0	7	0	0
Mymt Flow	475	3	2	742	6	6
IVIVIII( I IOW	410	3		172	U	U
Major/Minor I	Major1	N	Major2	I	Minor1	
Conflicting Flow All	0	0	478	0	1223	477
Stage 1	-	-	-	-	477	-
Stage 2	-	-	-	-	746	-
Critical Hdwy	-	-	4.1	-	6.4	6.2
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy	-	-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver	-	-	1095	-	200	592
Stage 1	_	-	-	_	629	_
Stage 2	_	_	_	-	472	-
Platoon blocked, %	_	_		_		
Mov Cap-1 Maneuver	_	_	1095	_	200	592
Mov Cap-2 Maneuver	_	_	-	_	200	-
Stage 1	_	_	_	_	629	_
Stage 2	_		_	_	471	_
Stage 2					4/1	
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		17.5	
HCM LOS					С	
N.C 1 /N.C		UDL 4	ГРТ	EDD	MDI	MOT
Minor Lane/Major Mvm	it f	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		299	-	-	1095	-
HCM Lane V/C Ratio		0.04	-	-	0.002	-
HCM Control Delay (s)		17.5	-	-	8.3	-
HCM Lane LOS HCM 95th %tile Q(veh)		0.1	-	-	A 0	-